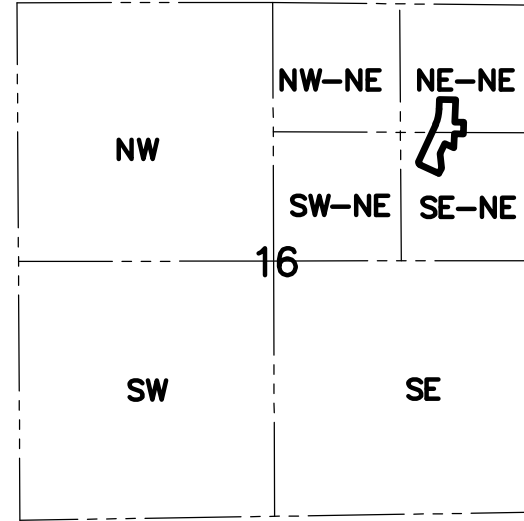


OAK MEADOW RESERVE

BEING A PART OF LOT 115, FIRST ADDITION TO OAK MEADOW AS RECORDED IN VOLUME 58-068B OF PLATS ON PAGE 355, AS DOCUMENT No. 4014340, DANE COUNTY REGISTRY, LOCATED IN THE NE $\frac{1}{4}$ -NE $\frac{1}{4}$ AND THE SE $\frac{1}{4}$ -NE $\frac{1}{4}$, ALL IN SECTION 16, TOWNSHIP 06 NORTH, RANGE 09 EAST, CITY OF FITCHBURG, DANE COUNTY, WISCONSIN.



LOCATION SKETCH
(NOT TO SCALE)
SECTION 16, T06N, R09E
CITY OF FITCHBURG, DANE COUNTY, WI

CORPORATE OWNER'S CERTIFICATE

[OWNER NAME], A LIMITED LIABILITY COMPANY DULY ORGANIZED AND EXISTING UNDER AND BY VIRTUE OF THE LAWS OF THE STATE OF WISCONSIN, AS AN OWNER, DOES HEREBY CERTIFY THAT SAID LIMITED LIABILITY COMPANY HAS CAUSED THE LANDS DESCRIBED ON THIS PLAT TO BE SURVEYED, DIVIDED, MAPPED AND DEDICATED AS REPRESENTED HEREON, AND DOES FURTHER CERTIFY THAT THIS PLAT IS REQUIRED BY S.236.10 OR S. 236.12 TO BE SUBMITTED TO THE FOLLOWING FOR APPROVAL OR OBJECTION:

COMMON COUNCIL, CITY OF FITCHBURG
DANE COUNTY ZONING AND LAND REGULATION COMMITTEE
STATE OF WISCONSIN-DEPARTMENT OF ADMINISTRATION

IN WITNESS WHEREOF, THE SAID [ENTITY NAME] HAS CAUSED THESE PRESENTS TO BE SIGNED BY [OWNER NAME], ITS MANAGER, AND ITS CORPORATE SEAL TO BE HEREUNTO AFFIXED ON THIS ___ DAY OF _____, 2024.

[ENTITY NAME]

[OWNER NAME]
MANAGER

STATE OF WISCONSIN)ss
COUNTY OF DANE)ss

PERSONALLY CAME BEFORE ME THIS _____ DAY OF _____, 2023, THE ABOVE NAMED [OWNER NAME], MANAGER OF THE ABOVE NAMED [ENTITY NAME], TO ME KNOWN TO BE THE PERSON WHO EXECUTED THE FOREGOING INSTRUMENT, AND ACKNOWLEDGED THE SAME.

NOTARY PUBLIC, STATE OF WISCONSIN

MY COMMISSION EXPIRES

CONSENT OF MORTGAGEE

[NAME], MORTGAGEE OF PART OF THE ABOVE DESCRIBED LAND, HEREBY CONSENTS TO THE SURVEYING, DIVIDING, MAPPING AND DEDICATION OF THE LAND DESCRIBED ON THIS PLAT, AND DOES HEREBY CONSENT TO THE ABOVE CERTIFICATE OF [NAME], AS AN OWNER.

WITNESS THE HAND AND SEAL OF [NAME], MORTGAGEE, THIS _____ DAY OF _____, 2024.

IN THE PRESENCE OF:

(SIGN NAME HERE)

(PRINT NAME AND TITLE HERE)

STATE OF WISCONSIN)ss
COUNTY OF DANE)ss

PERSONALLY CAME BEFORE ME THIS _____ DAY OF _____, 2024, THE ABOVE NAMED OFFICER OF THE ABOVE NAMED [NAME], TO ME KNOWN TO BE THE PERSON WHO EXECUTED THE FOREGOING INSTRUMENT, AND ACKNOWLEDGED THE SAME.

NOTARY PUBLIC, STATE OF WISCONSIN

MY COMMISSION EXPIRES

CERTIFICATE OF REGISTER OF DEEDS

RECEIVED FOR RECORDING THIS _____ DAY OF _____, 2024, AT _____ O'CLOCK, _____ M. AND

RECORDED IN VOLUME _____ OF PLATS ON PAGES _____ THROUGH _____ AS DOCUMENT

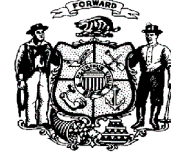
NUMBER _____

KRISTI CHLEBOWSKI

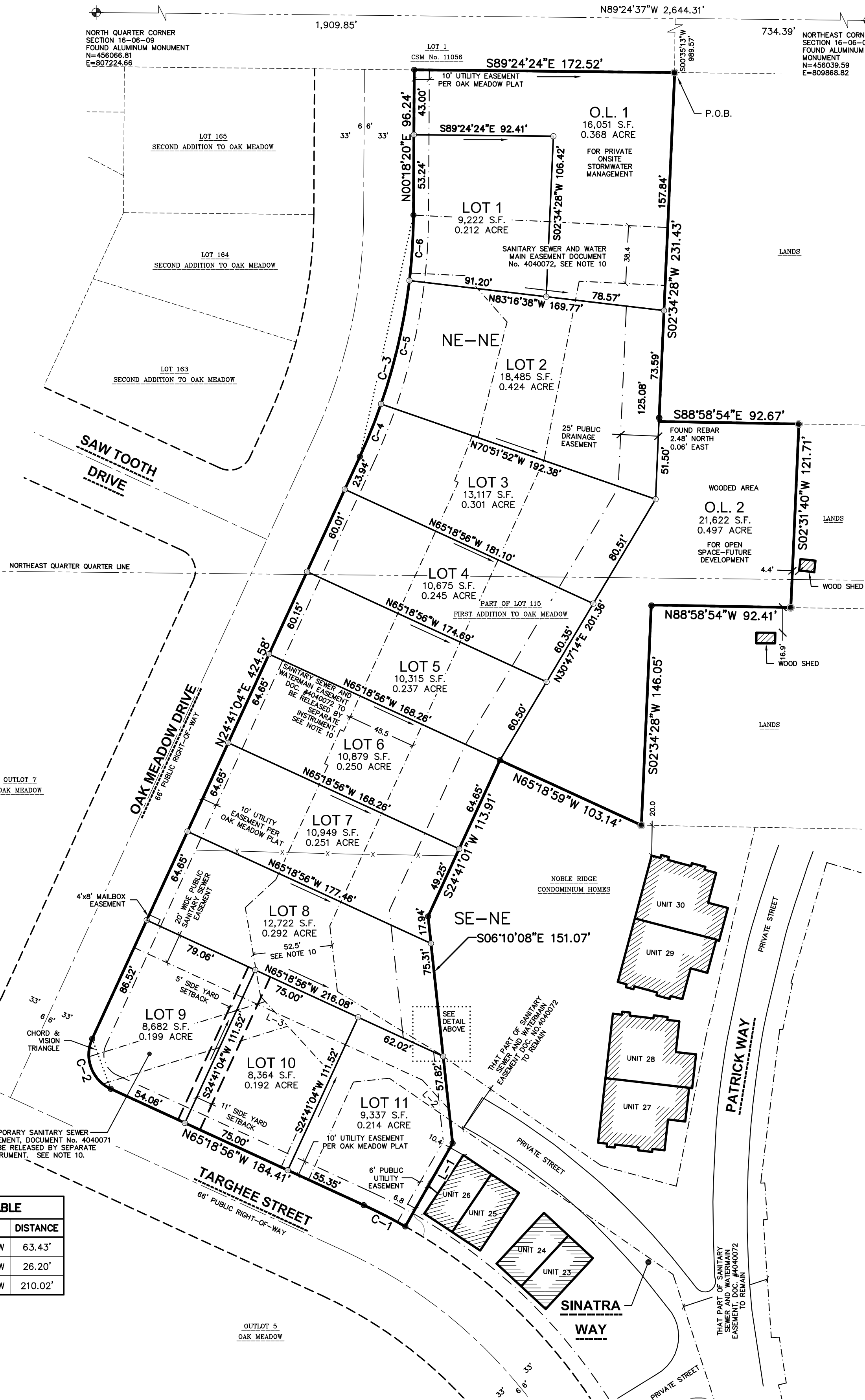
REGISTER OF DEEDS, DANE COUNTY

There are no objections to this plat with respect to Secs. 236.15, 236.16, 236.20 and 236.21(1) and (2), Wis Stats. as provided by s. 236.12, Wis. Stats.

Certified _____, 20 _____



Department of Administration



LEGEND

- GOVERNMENT CORNER
- 3/4" REBAR FOUND
- 1-1/4" REBAR FOUND
- 1" IRON PIPE FOUND
- 3/4" x 24" REBAR SET (1.50 LBS/LF)
- PARCEL BOUNDARY
- LOT LINES
- RIGHT-OF-WAY LINE
- CENTERLINE
- - - - - PLATTED LOT LINE
- - - - - SECTION LINE
- - - - - DRAINAGE EASEMENT LINE
- - - - - UTILITY EASEMENT LINE
- - - - - EASEMENT LINE
- - - - - CHORD LINE
- - - - - CONCRETE CURB & GUTTER
- - - - - FENCE LINE
- - - - - BUILDING
- () DENOTES RECORDED AS MEASUREMENTS DEPICTING THE SAME LINE ON THE GROUND AS RETRACED BY THIS SURVEY
- ARROWS INDICATE THE DIRECTION OF SURFACE DRAINAGE WITHIN INDIVIDUAL PROPERTY LINES. SAID DRAINAGE SHALL BE GRADED WITH THE CONSTRUCTION OF EACH PRINCIPAL STRUCTURE AND MAINTAINED BY THE LOT OWNER UNLESS MODIFIED WITH THE APPROVAL OF THE CITY OF FITCHBURG ENGINEER. THE FINAL GRADE SHALL NOT BE ALTERED BY MORE THAN 6 INCHES WITHOUT PRIOR WRITTEN CONSENT OF THE CITY OF FITCHBURG ENGINEER AND THE APPROPRIATE UTILITY COMPANY OR COMPANIES.

NOTES

1. FIELD WORK PERFORMED ON NOVEMBER 30, 2023.
2. BEARINGS FOR THIS SURVEY AND MAP ARE REFERENCED TO GRID NORTH OF THE WISCONSIN COUNTY COORDINATE SYSTEM, DANE COUNTY. THE NORTH LINE OF THE NORTHEAST QUARTER OF SECTION 16, TOWNSHIP 06 NORTH, RANGE 09 EAST, BEARS S89°24'37".
3. OUTLOT 1 AND OUTLOT 2 SHALL BE PRIVATELY OWNED AND MAINTAINED.
4. PROPOSED ZONING FOR THIS PLAT IS "LOW TO MEDIUM DENSITY RESIDENTIAL (R-LM)".
5. UTILITY EASEMENTS - NO POLES OR BURIED CABLES ARE TO BE PLACED SUCH THAT THE INSTALLATION WOULD DISTURB ANY SURVEY STAKE OR OBSTRUCT VISION ALONG ANY LOT LINE OR STREET LINE.
6. THE DISTURBANCE OF A LOT CORNER BY ANYONE IS A VIOLATION OF SECTION 236.32 OF WISCONSIN STATUTES.
7. UTILITY EASEMENTS AS HEREIN SET FORTH ARE FOR THE USE OF PRIVATE PUBLIC UTILITIES HAVING THE RIGHT TO SERVE THIS PLAT. THE USE OF THESE EASEMENTS IS NOT TO BE EXTENDED TO PRIVATE OR PUBLIC PRIVATE UTILITIES SUCH AS WATER, SANITARY SEWER AND STORM WATER UTILITIES, EXCEPT WHERE SPECIFICALLY NOTED ON THE PLAT.
8. FINAL GRADE WITHIN PUBLIC UTILITY EASEMENTS SHALL NOT BE ALTERED BY MORE THAN 6 INCHES WITHOUT THE WRITTEN CONSENT OF ALL UTILITIES INVOLVED.
9. THE 25' PROPOSED DRAINAGE EASEMENT ALONG LOTS 2 THROUGH 8, AND THE STORMWATER FACILITY ON OUTLOT 1, ARE FOR THE BENEFIT OF THE LOTS ASSOCIATED WITH PLAT OF "OAK MEADOW RESERVE".
10. THAT PORTION OF THE "SANITARY SEWER AND WATERMAIN EASEMENT", RECORDED IN DOCUMENT NO. 4040072, THAT FALLS INSIDE OF THE PLAT OF "OAK MEADOW RESERVE" SHALL BE RELEASED BY SEPARATE INSTRUMENT. THE TEMPORARY SANITARY SEWER EASEMENT RECORDED AS DOCUMENT NO. 4040071 SHALL BE RELEASED BY SEPARATE INSTRUMENT.

CERTIFICATE OF CITY OF FITCHBURG CLERK

RESOLVED THAT THIS PLAT KNOWN AS "OAK MEADOW RESERVE", LOCATED IN CITY OF FITCHBURG, WAS HEREBY APPROVED BY RESOLUTION No. _____, FILE No. _____, ADOPTED ON THE _____ DAY OF _____, 2024, AND FURTHER RESOLVED THAT THE CONDITIONS OF SAID APPROVAL WERE FULFILLED ON THE _____ DAY OF _____, 2024, AND THAT SAID RESOLUTION FURTHER PROVIDED FOR THE ACCEPTANCE OF THOSE LANDS AND RIGHTS DEDICATED BY SAID "OAK MEADOW RESERVE" FOR PUBLIC USE.

TRACY OLDENBURG, CITY CLERK

DATE

CERTIFICATE OF CITY TREASURER

I, MISTY F. DODGE, BEING DULY ELECTED, QUALIFIED, AND ACTING CITY TREASURER OF THE CITY OF FITCHBURG, DO HEREBY CERTIFY THAT IN ACCORDANCE WITH THE RECORDS IN MY OFFICE, THERE ARE NO UNPAID TAXES OR UNPAID SPECIAL ASSESSMENTS AS OF THE DAY OF _____, 2024, ON ANY OF THE LANDS INCLUDED IN THE PLAT OF "OAK MEADOW RESERVE".

MISTY F. DODGE, TREASURER, CITY OF FITCHBURG

DATE

CERTIFICATE OF COUNTY TREASURER

I, ADAM GALLAGHER, BEING THE DULY APPOINTED, QUALIFIED, AND ACTING TREASURER OF THE COUNTY OF DANE, DO HEREBY CERTIFY THAT THE RECORDS IN MY OFFICE SHOW NO UNREDEEMED TAX SALES AND UNPAID TAXES OR SPECIAL ASSESSMENTS AS OF THE DAY OF _____, 2024, AFFECTING THE LANDS INCLUDED IN THE PLAT OF "OAK MEADOWS RESERVE".

ADAM GALLAGHER
TREASURER, DANE COUNTY

DATE

SURVEYOR'S CERTIFICATE

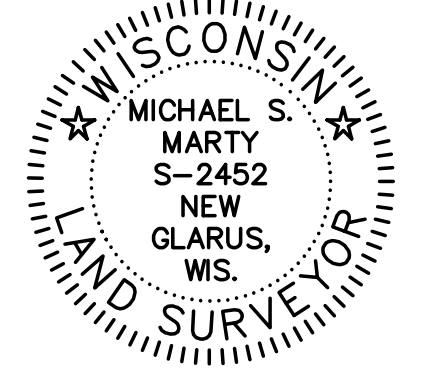
I, MICHAEL S. MARTY, WISCONSIN PROFESSIONAL LAND SURVEYOR NO. S-2452, HEREBY CERTIFY THAT IN FULL COMPLIANCE WITH THE PROVISIONS OF CHAPTER 236 OF THE WISCONSIN STATE STATUTES AND THE LAND DIVISION AND SUBDIVISION REGULATIONS OF THE CITY OF FITCHBURG, AND BY THE DIRECTION OF THOMPSON ENGINEERING, LLC, I HAVE SURVEYED, DIVIDED AND MAPPED THE FINAL PLAT OF OAK MEADOW RESERVE IN THE CITY OF FITCHBURG, DANE COUNTY, WISCONSIN, AND THAT SUCH PLAT IS A CORRECT REPRESENTATION OF THE EXTERIOR BOUNDARIES OF THE LANDS SURVEYED AND THE DIVISION THEREOF, DESCRIBED AS FOLLOWS:

BEING A PART OF LOT 115, FIRST ADDITION TO OAK MEADOW AS RECORDED IN VOLUME 58-068B OF PLATS ON PAGE 355, AS DOCUMENT NUMBER 4014340, DANE COUNTY REGISTRY, LOCATED IN THE NE $\frac{1}{4}$ -NE $\frac{1}{4}$ AND THE SE $\frac{1}{4}$ -NE $\frac{1}{4}$, ALL IN SECTION 16, TOWNSHIP 06 NORTH, RANGE 09 EAST, CITY OF FITCHBURG, DANE COUNTY, WISCONSIN, MORE FULLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE NORTHEAST CORNER OF SAID SECTION 16; THENCE N89°24'37"W ALONG THE NORTH LINE OF THE NORTHEAST QUARTER (NE $\frac{1}{4}$) OF SAID SECTION 16, 734.39 FEET; THENCE S00°35'13"W, 989.57 FEET TO THE NORTHEAST CORNER OF SAID LOT 115, FIRST ADDITION TO OAK MEADOW AND THE POINT OF BEGINNING; THENCE ALONG THE BOUNDARY OF SAID LOT 115 FOR THE NEXT 5 COURSES: 1- THENCE S02°34'28"W, 231.43 FEET; 2- THENCE S88°58'54"E, 92.67 FEET; 3- THENCE S02°31'40"W, 121.71 FEET; 4- THENCE N88°58'54"W, 92.41 FEET; 5- THENCE S02°34'28"W, 146.05 FEET TO A NORTHERLY CORNER OF NOBEL RIDGE CONDOMINIUM HOMES FIRST ADDENDUM, AS RECORDED IN VOLUME 7-075B OF CONDOMINIUM PLATS, ON PAGES 1-7, AS DOCUMENT NUMBER 5448308, DANE COUNTY REGISTRY; THENCE ALONG THE BOUNDARY OF SAID CONDOMINIUM FOR THE NEXT FOUR (4) COURSES: 1- THENCE N65°18'59"W, 103.14 FEET; 2- THENCE S24°41'01"W, 113.91 FEET; 3- THENCE S09°10'08"E, 153.07 FEET; 4- THENCE S29°34'15"W, 63.43 FEET TO A POINT ON THE NORTHEASTLY RIGHT-OF-WAY LINE OF TARGHEE STREET, SAID POINT ALSO BEING A POINT OF NON-TANGENTIAL CURVATURE; THENCE 30.96 FEET ALONG SAID RIGHT-OF-WAY AND THE ARC OF A CURVE TO THE LEFT, HAVING A RADIUS OF 363.00 FEET, A CENTRAL ANGLE OF 04°33'11", AND A CHORD BEARING N62°52'20"W, 30.95 FEET; THENCE N65°18'56"W ALONG SAID RIGHT-OF-WAY, 184.41 FEET TO A POINT OF CURVATURE; THENCE 39.27 FEET ALONG SAID RIGHT-OF-WAY AND THE ARC OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 25.00 FEET, A CENTRAL ANGLE OF 90°00'00", AND A CHORD BEARING N20°18'56"W, 35.36 FEET TO THE SOUTHEASTLY RIGHT-OF-WAY OF OAK MEADOW DRIVE; THENCE N24°41'04"E ALONG SAID SOUTHEASTLY RIGHT-OF-WAY, 424.58 FEET TO A POINT OF CURVATURE; THENCE 165.09 FEET ALONG SAID RIGHT-OF-WAY AND THE ARC OF A CURVE TO THE LEFT, HAVING A RADIUS OF 388.00 FEET, A CENTRAL ANGLE OF 24°22'44", AND A CHORD BEARING N12°29'42"E, 163.85 FEET; THENCE N06°43'22"E ALONG SAID RIGHT-OF-WAY, 96.24 FEET TO THE NORTHWEST CORNER OF SAID LOT 115; THENCE S89°24'24"E ALONG THE NORTH LINE OF SAID LOT 115, 172.52 FEET TO THE NORTHEAST CORNER OF SAID LOT 115 AND THE POINT OF BEGINNING. SAID DESCRIPTION CONTAINS 160,416 SQ. FT. OR 3.663 ACRES MORE OR LESS.

MICHAEL S. MARTY, S-2452
PROFESSIONAL LAND SURVEYOR

DATE

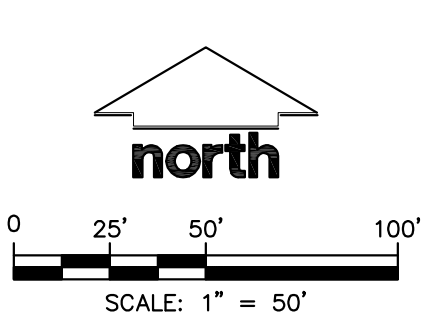


SURVEYED BY:
JSD PROFESSIONAL SERVICES, INC.
507 W VERONA AVE, STE 500
VERONA, WISCONSIN 53593
(608) 848-5060

SURVEYED FOR:
THOMPSON ENGINEERING LLC
918 DEMING WAY, FLOOR 3
MADISON, WI 53717
(608) 827-5700

CURVE TABLE						
CURVE	ARC LENGTH	RADIUS	DELTA	CHORD BEARING	CHORD	TANGENT IN TANGENT OUT
C-1	30.96'	363.00'	4°53'11"	N62°52'20"W	30.95'	N60°25'45"W N65°18'56"E
C-2	39.27'	25.00'	90°00'00"	N20°18'56"W	35.36'	S24°41'04"W S65°18'56"E
()	39.27'	25.00'	90°00'00"	N20°18'56"W	35.36'	- - - - -
C-3	165.09'	388.00'	24°22'44"	N12°29'42"E	163.85'	N24°41'04"E N00°18'20"E
()	165.09'	388.00'	24°22'44"	N12°29'42"E	163.85'	- - - - -
C-4	37.58'	388.00'	5°32'57"	N21°54'36"E	37.56'	N24°41'04"E N19°08'08"E
C-5	84.06'	388.00'	12°24'46"	N12°55'45"E	83.89'	N19°08'08"E N06°43'22"E
C-6	43.46'	388.00'	6°25'02"	N03°30'51"E	43.43'	N06°43'22"E N00°18'20"E

LINE TABLE		
LINE	BEARING	DISTANCE
L-1	S29°34'15"W	63.43'
L-2	N42°19'30"W	26.20'
L-3	N65°18'56"W	210.02'



PROJECT LOCATION:
SECTION 16
TOWNSHIP 06 NORTH
RANGE 09 EAST
CITY OF FITCHBURG
DANE COUNTY, WI

JSD CREATE THE VISION TELL THE STORY
jsdinc.com

OAK MEADOW RESERVE

PROJECT NO: 23-13509 SURVEYED BY: CWD INITIAL SUBMITTAL DATE: JUNE 18, 2024
FIELDBOOK/PG: - DRAWN BY: CWD/MSM REVISION RESUBMITTAL DATE: -
REVISION RESUBMITTAL DATE: -
REVISION RESUBMITTAL DATE: -

SHEET 1 OF 1

File: \\jsdinc\projects\2023\2313509\DWG\Survey_Sheet1\2313509_F-Plat.dwg Layout: Sheet 1 User: mmarty Plotted: Jun 18, 2024 - 2:43pm

GENERAL NOTES

- REFER TO THE EXISTING CONDITIONS SURVEY FOR EXISTING CONDITIONS NOTES AND LEGENDS.
- ALL WORK IN THE RIGHT-OF-WAY AND/OR PUBLIC EASEMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, JURISDICTIONAL SPECIFICATIONS, AND APPROVED BY THE JURISDICTION HAVING AUTHORITY.
- EXISTING GRADE SPOT ELEVATIONS SHOWN FOR INFORMATIONAL PURPOSES. DURING CONSTRUCTION MATCH EXISTING GRADES AT CONSTRUCTION LIMITS.
- NO SITE GRADING OUTSIDE OR DOWNSLOPE OF PROPOSED SILT FENCE LOCATION. NO LAND DISTURBANCE BEYOND PROPERTY LINES.
- JSD SHALL BE HELD HARMLESS AND DOES NOT WARRANT ANY DEVIATIONS BY THE OWNER/CONTRACTOR FROM THE APPROVED CONSTRUCTION PLANS THAT MAY RESULT IN DISCIPLINARY ACTIONS BY ANY OR ALL REGULATORY AGENCIES.
- CONTRACTOR SHALL RESTORE ALL BUILDINGS, PAVEMENT, PIPES, SLOPES, AND STRUCTURES DAMAGED BY THE CONTRACTOR TO PRE-EXISTING OR BETTER CONDITIONS.
- THE RIGHT-OF-WAY IS THE SOLE JURISDICTION OF THE JURISDICTIONAL AUTHORITY AND IS SUBJECT TO CHANGE AT ANY TIME.
CONTRACTOR SHALL BE RESPONSIBLE FOR SITE SAFETY DURING THE CONSTRUCTION OF THESE IMPROVEMENTS.
- ANY REFERENCES TO THE TERMS OR ENTITY ABBREVIATIONS IN THE FOLLOWING NOTES AND SPECIFICATIONS SHALL BE UNDERSTOOD AS FOLLOWS:
 - "JURISDICTION" - THE LOCAL GOVERNMENTAL AGENCY (I.E., CITY, VILLAGE, TOWN, COUNTY, STATE, OR UTILITY SERVICE PROVIDER) HAVING AUTHORITY.
 - "STATE HIGHWAY SPECIFICATIONS" - STATE OF WISCONSIN STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION, CURRENT EDITION AND SUPPLEMENTS
 - "STANDARD SPECIFICATIONS" - STANDARD SPECIFICATIONS FOR SEWER AND WATER CONSTRUCTION IN WISCONSIN, CURRENT EDITION AND SUPPLEMENTS
 - WISCONSIN DEPARTMENT OF TRANSPORTATION - "WISDOT"
 - WISCONSIN DEPARTMENT OF NATURAL RESOURCES - "WDNR"
 - DEPARTMENT OF SAFETY AND PROFESSIONAL SERVICES - "DSPS" OR "SPS"

DEMOLITION NOTES

- THIS PLAN INDICATES ITEMS ON THE PROPERTY INTENDED FOR DEMOLITION BASED ON THE CURRENT SITE DESIGN THAT HAVE BEEN IDENTIFIED BY A REASONABLE OBSERVATION OF THE EXISTING CONDITIONS THROUGH FIELD SURVEY RECONNAISSANCE, "DIGGERS HOTLINE" LOCATION, AND GENERAL "STANDARD OF CARE" THERE MAY BE ADDITIONAL ITEMS THAT CAN NOT BE IDENTIFIED BY A REASONABLE ABOVEGROUND OBSERVATION, OF WHICH THE ENGINEER WOULD HAVE NO KNOWLEDGE OR MAY BE A PART OF ANOTHER DESIGN DISCIPLINE. IT IS THE CONTRACTOR'S/BIDDER'S RESPONSIBILITY TO REVIEW THE PLANS, INSPECT THE SITE, AND PROVIDE THEIR OWN DUE DILIGENCE TO INCLUDE IN THEIR BID WHAT ADDITIONAL ITEMS, IN THEIR OPINION, MAY BE NECESSARY FOR DEMOLITION. ANY ADDITIONAL ITEMS IDENTIFIED BY THE CONTRACTOR/BIDDER SHALL BE IDENTIFIED IN THE BID AND REPORTED TO THE ENGINEER OF RECORD. JSD TAKES NO RESPONSIBILITY FOR ITEMS ON THE PROPERTY THAT COULD NOT BE LOCATED BY A REASONABLE OBSERVATION OF THE PROPERTY OR OF WHICH THEY WOULD HAVE NO KNOWLEDGE.
- CONTRACTOR SHALL KEEP ALL STREETS AND PRIVATE DRIVES FREE AND CLEAR OF ALL CONSTRUCTION-RELATED DIRT, DUST, AND DEBRIS.
- ALL TREES WITHIN THE CONSTRUCTION LIMITS SHALL BE REMOVED UNLESS SPECIFICALLY CALLED OUT FOR PROTECTION. ALL TREES TO BE REMOVED SHALL BE REMOVED IN THEIR ENTIRETY. STUMPS MAY BE GROUND TO PROPOSED SUBGRADE IN GRASSED AREAS ONLY UNLESS DIRECTED BY ENGINEER.
- ALL LIGHT POLES TO BE REMOVED SHALL BE REMOVED IN THEIR ENTIRETY, INCLUDING BASE AND ALL APPURTENANCES. SALVAGE FOR RELOCATION. COORDINATE RELOCATION AND/OR ABANDONMENT OF ALL ELECTRIC LINES WITH ELECTRICAL ENGINEER AND OWNER PRIOR TO DEMOLITION.
- ABANDONED/REMOVED ITEMS SHALL BE LEGALLY DISPOSED OF OFFSITE UNLESS OTHERWISE NOTED.
- CONTRACTOR TO REPLACE ALL SIDEWALK AND CURB AND GUTTER ABUTTING THE PROPERTIES THAT WERE DAMAGED BY THE CONSTRUCTION.
- PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL BE RESPONSIBLE TO:
 - EXAMINE ALL SITE CONDITIONS RELATIVE TO THE CONDITIONS INDICATED ON THE ENGINEERING DRAWINGS. ANY DISCREPANCIES ARE TO BE REPORTED IMMEDIATELY TO THE ENGINEER AND RESOLVED PRIOR TO THE START OF CONSTRUCTION.
 - VERIFY UTILITY ELEVATIONS AND NOTIFY ENGINEER OF ANY DISCREPANCIES, NO WORK SHALL BE PERFORMED UNTIL THE DISCREPANCIES ARE RESOLVED.
 - NOTIFY ALL UTILITIES OWNERS PRIOR TO THE REMOVAL OF ANY UNDERGROUND UTILITIES.
 - NOTIFY THE DESIGN ENGINEER AND LOCAL CONTROLLING MUNICIPALITY 48 HOURS PRIOR TO THE START OF CONSTRUCTION TO ARRANGE FOR APPROPRIATE CONSTRUCTION INSPECTION.
- ANY UTILITIES THAT ARE DAMAGED BY THE CONTRACTORS SHALL BE REPAIRED TO THE OWNER'S SATISFACTION AT THE CONTRACTOR'S EXPENSE.
- CONTRACTOR SHALL COORDINATE PRIVATE UTILITY REMOVAL/ABANDONMENT AND NECESSARY RELOCATION WITH RESPECTIVE UTILITY COMPANY. CONTRACTOR SHALL COORDINATE WITH PRIVATE UTILITY COMPANIES PRIOR TO CONSTRUCTION.
- ALL DEMOLITION SHALL BE IN ACCORDANCE WITH THE APPROVED JURISDICTIONS RECYCLING PLAN.
- ANY CONTAMINATED SOILS ENCOUNTERED SHALL BE REMOVED IN ACCORDANCE WITH FEDERAL AND STATE REGULATIONS TO AN APPROPRIATE AND APPROVED LANDFILL.
- ALL EXISTING UTILITIES SHALL BE FIELD LOCATED AND CLEARLY MARKED BY CONTRACTOR PRIOR TO ANY EXCAVATION. CONTRACTOR SHALL NOTIFY ENGINEER IMMEDIATELY IF ANY DISCREPANCIES OCCUR IN THE LOCATION SHOWN OR PROPOSED IMPROVEMENTS IMPACTING EXISTING UTILITY LINE LOCATION(S). CONTRACTOR SHALL BE RESPONSIBLE FOR CONDUCTING UTILITY LINE OPENINGS (ULO) TO CONFIRM LOCATIONS OR ELEVATIONS, AS REQUESTED BY THE ENGINEER.
- SEWER ABANDONMENT SHALL BE IN ACCORDANCE WITH SECTION 3.2.24 OF THE STANDARD SPECIFICATIONS AND JURISDICTIONAL SPECIFICATIONS.
- WATER ABANDONMENT SHALL BE IN ACCORDANCE WITH SECTION 4.14.0 OF THE STANDARD SPECIFICATIONS AND JURISDICTIONAL SPECIFICATIONS.
- ALL PERIMETER EROSION CONTROL DEVICES SHALL BE INSTALLED PRIOR TO THE START OF DEMOLITION ACTIVITIES. CONTRACTOR SHALL KEEP ALL STREETS AND PAVEMENTS FREE AND CLEAR OF ALL CONSTRUCTION RELATED DIRT, DUST, AND DEBRIS.
- BUILDING REMOVALS SHALL BE PERFORMED BY A QUALIFIED CONTRACTOR. CONTRACTOR SHALL FOLLOW ALL DEMOLITION REGULATIONS, DISCONNECT ALL UTILITIES, OBTAIN ALL APPLICABLE PERMITS, AND DISPOSE OF ALL BUILDING MATERIALS IN APPROPRIATE AND APPROVED LANDFILLS. DEMOLISHED MATERIALS SHALL NOT BE BURNED OR BURIED ONSITE.
- CONTRACTOR SHALL REMOVE EXISTING UTILITY PIPE OR PROVIDE PIPE BACKFILL AFTER REMOVAL OF EXISTING UTILITIES WITHIN BUILDING FOOTPRINT USING "LOW DENSITY CONCRETE/FLOWABLE FILL".
- RESTORATION OF THE EXISTING ROADWAY RIGHT-OF-WAYS ARE CONSIDERED INCIDENTAL AND SHALL BE PART OF THE COST OF THE UNDERGROUND IMPROVEMENTS. DEMOLITION, AND REMOVAL. THIS INCLUDES CURB AND GUTTER, SIDEWALK, TOPSOIL, SEEDING, AND MULCHING.

- CONTRACTOR SHALL COORDINATE PRIVATE UTILITY REMOVAL/ABANDONMENT AND NECESSARY RELOCATION WITH RESPECTIVE UTILITY COMPANY. CONTRACTOR SHALL COORDINATE WITH PRIVATE UTILITY COMPANIES PRIOR TO CONSTRUCTION.
- ALL DEMOLITION SHALL BE IN ACCORDANCE WITH THE APPROVED JURISDICTIONS RECYCLING PLAN.
- ANY CONTAMINATED SOILS ENCOUNTERED SHALL BE REMOVED IN ACCORDANCE WITH FEDERAL AND STATE REGULATIONS TO AN APPROPRIATE AND APPROVED LANDFILL.
- ALL EXISTING UTILITIES SHALL BE FIELD LOCATED AND CLEARLY MARKED BY CONTRACTOR PRIOR TO ANY EXCAVATION. CONTRACTOR SHALL NOTIFY ENGINEER IMMEDIATELY IF ANY DISCREPANCIES OCCUR IN THE LOCATION SHOWN OR PROPOSED IMPROVEMENTS IMPACTING EXISTING UTILITY LINE LOCATION(S). CONTRACTOR SHALL BE RESPONSIBLE FOR CONDUCTING UTILITY LINE OPENINGS (ULO) TO CONFIRM LOCATIONS OR ELEVATIONS, AS REQUESTED BY THE ENGINEER.
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- RESTORATION OF THE EXISTING ROADWAY RIGHT-OF-WAYS ARE CONSIDERED INCIDENTAL AND SHALL BE PART OF THE COST OF THE UNDERGROUND IMPROVEMENTS. DEMOLITION, AND REMOVAL. THIS INCLUDES CURB AND GUTTER, SIDEWALK, TOPSOIL, SEEDING, AND MULCHING.

- CONTRACTOR SHALL COORDINATE PRIVATE UTILITY REMOVAL/ABANDONMENT AND NECESSARY RELOCATION WITH RESPECTIVE UTILITY COMPANY. CONTRACTOR SHALL COORDINATE WITH PRIVATE UTILITY COMPANIES PRIOR TO CONSTRUCTION.
- ALL DEMOLITION SHALL BE IN ACCORDANCE WITH THE APPROVED JURISDICTIONS RECYCLING PLAN.
- ANY CONTAMINATED SOILS ENCOUNTERED SHALL BE REMOVED IN ACCORDANCE WITH FEDERAL AND STATE REGULATIONS TO AN APPROPRIATE AND APPROVED LANDFILL.
- ALL EXISTING UTILITIES SHALL BE FIELD LOCATED AND CLEARLY MARKED BY CONTRACTOR PRIOR TO ANY EXCAVATION. CONTRACTOR SHALL NOTIFY ENGINEER IMMEDIATELY IF ANY DISCREPANCIES OCCUR IN THE LOCATION SHOWN OR PROPOSED IMPROVEMENTS IMPACTING EXISTING UTILITY LINE LOCATION(S). CONTRACTOR SHALL BE RESPONSIBLE FOR CONDUCTING UTILITY LINE OPENINGS (ULO) TO CONFIRM LOCATIONS OR ELEVATIONS, AS REQUESTED BY THE ENGINEER.
- SEWER ABANDONMENT SHALL BE IN ACCORDANCE WITH SECTION 3.2.24 OF THE STANDARD SPECIFICATIONS AND JURISDICTIONAL SPECIFICATIONS.
- WATER ABANDONMENT SHALL BE IN ACCORDANCE WITH SECTION 4.14.0 OF THE STANDARD SPECIFICATIONS AND JURISDICTIONAL SPECIFICATIONS.
- ALL PERIMETER EROSION CONTROL DEVICES SHALL BE INSTALLED PRIOR TO THE START OF DEMOLITION ACTIVITIES. CONTRACTOR SHALL KEEP ALL STREETS AND PAVEMENTS FREE AND CLEAR OF ALL CONSTRUCTION RELATED DIRT, DUST, AND DEBRIS.
- BUILDING REMOVALS SHALL BE PERFORMED BY A QUALIFIED CONTRACTOR. CONTRACTOR SHALL FOLLOW ALL DEMOLITION REGULATIONS, DISCONNECT ALL UTILITIES, OBTAIN ALL APPLICABLE PERMITS, AND DISPOSE OF ALL BUILDING MATERIALS IN APPROPRIATE AND APPROVED LANDFILLS. DEMOLISHED MATERIALS SHALL NOT BE BURNED OR BURIED ONSITE.
- CONTRACTOR SHALL REMOVE EXISTING UTILITY PIPE OR PROVIDE PIPE BACKFILL AFTER REMOVAL OF EXISTING UTILITIES WITHIN BUILDING FOOTPRINT USING "LOW DENSITY CONCRETE/FLOWABLE FILL".
- RESTORATION OF THE EXISTING ROADWAY RIGHT-OF-WAYS ARE CONSIDERED INCIDENTAL AND SHALL BE PART OF THE COST OF THE UNDERGROUND IMPROVEMENTS. DEMOLITION, AND REMOVAL. THIS INCLUDES CURB AND GUTTER, SIDEWALK, TOPSOIL, SEEDING, AND MULCHING.

CONSTRUCTION SEQUENCING

- INSTALL PERIMETER SILT FENCE, WATTLES, INLET PROTECTION, AND CONSTRUCTION ENTRANCES. (9/3/2024)
- STRIP AND STOCKPILE TOPSOIL AND INSTALL SILT FENCE AROUND PERIMETER OF STOCKPILE. (9/3/2024)
- INSTALL UTILITY PIPING AND STRUCTURES, IMMEDIATELY INSTALL INLET PROTECTION.
- ROUGH GRADE RETENTION POND AND INSTALL POND OUTLET.
- CONDUCT ROUGH GRADING EFFORTS AND INSTALL CHECK DAMS WITHIN DRAINAGE DITCHES.
- COMPLETE FINAL GRADING, INSTALLATION OF GRAVEL BASE COURSES, PLACEMENT OF CURBS, PAVEMENTS, WALKS, ETC.
- PLACE TOPSOIL AND IMMEDIATELY STABILIZE DISTURBED AREAS WITH EROSION CONTROL MEASURES AS INDICATED ON PLANS. (9/5/2025)
- RESTORE RETENTION POND /FINAL GRADE RETENTION POND PER PLAN REQUIREMENTS)
- EROSION CONTROLS SHALL NOT BE REMOVED UNTIL SITE IS FULLY STABILIZED OR 70% CONTIGUOUS VEGETATIVE COVER IS ESTABLISHED. (9/5/2025)

CONTRACTOR MAY MODIFY SEQUENCING AFTER ITEM NO. 1 AS NEEDED TO COMPLETE CONSTRUCTION IF EROSION CONTROLS ARE MAINTAINED IN ACCORDANCE WITH THE CONSTRUCTION SITE EROSION CONTROL REQUIREMENTS.

PAVING NOTES

- GENERAL:**
 - PAVING SHALL CONFORM TO STATE HIGHWAY SPECIFICATIONS, APPLICABLE JURISDICTIONAL SPECIFICATIONS, AND THE GEOTECHNICAL REPORT PREPARED BY INTERTEK PSI, ISSUE DATE 01/15/24. ALL REFERENCES TO THE "GEOTECHNICAL REPORT" SHALL BE UNDERSTOOD AS THE AFOREMENTIONED REPORT.
 - ALL PAVING DIMENSIONS ARE TO FACE OF CURB UNLESS SPECIFIED OTHERWISE.
 - ALL SPOT GRADES ARE TO EDGE OF PAVEMENT UNLESS SPECIFIED OTHERWISE.
 - SURFACE PREPARATION - NOTIFY ENGINEER/OWNER OF UNSATISFACTORY CONDITIONS. DO NOT BEGIN PAVING WORK UNTIL DEFICIENT SUBBASE AREAS HAVE BEEN CORRECTED AND ARE READY TO RECEIVE PAVING.
 - ANY REQUIRED REPLACEMENT OF PUBLIC CURB AND GUTTER, PAVEMENT, OR SIDEWALK SHALL MATCH EXISTING AND MEET JURISDICTIONAL REQUIREMENTS.
 - CRUSHED AGGREGATE BASE COURSE SPECIFICATIONS:**
 - THE TOP LAYER OF BASE COURSE SHALL CONFORM TO SECTIONS 301 AND 305 OF THE STATE HIGHWAY SPECIFICATIONS.
 - RECLAIMED OR RECYCLED ASPHALT MAY NOT BE USED AS CRUSHED AGGREGATE BASE COURSE UNLESS SPECIFICALLY APPROVED BY THE ENGINEER OF RECORD. USE OF ANY OTHER REPROCESSED OR BLENDED MATERIAL MUST FIRST BE APPROVED BY ENGINEER OF RECORD.
 - DO NOT PLACE BASE ON FROZEN FOUNDATIONS UNLESS THE ENGINEER APPROVES OTHERWISE.
 - DO NOT PLACE BASE ON FOUNDATIONS THAT ARE SOFT, SPONGY, OR COVERED BY ICE OR SNOW.
 - HOT MIXED ASPHALT (HMA) PAVING SPECIFICATIONS:**
 - THE PLACING, CONSTRUCTION, AND COMPOSITION OF THE BASE COURSE AND HMA SURFACE COURSE SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTIONS 450, 455, 460, AND 465 OF THE STATE HIGHWAY SPECIFICATIONS.
 - WEATHER LIMITATIONS:
 - DO NOT PLACE ASPHALTIC MIXTURE WHEN THE AIR TEMPERATURE IS APPROXIMATELY 3° ABOVE GRADE, IN SHADE, AND AWAY FROM ARTIFICIAL HEAT SOURCES IS LESS THAN 40°F UNLESS A VALID ENGINEER-ACCEPTED COLD WEATHER PAVING PLAN IS IN EFFECT.
 - PLACE ASPHALTIC MIXTURE ONLY ON A PREPARED, FIRM, AND COMPACTED BASE, FOUNDATION LAYER, OR EXISTING PAVEMENT SUBSTANTIALLY SURFACE-DRY AND FREE OF LOOSE AND FOREIGN MATERIAL. DO NOT PLACE OVER FROZEN SUBGRADE OR BASE, OR WHERE THE ROADBED IS UNSTABLE.
 - APPLY TACK COAT ONLY WHEN THE AIR TEMPERATURE IS 32°F OR MORE UNLESS THE ENGINEER APPROVES OTHERWISE IN WRITING.
 - ALL ASPHALT (BOTH UPPER AND LOWER LAYERS) SHALL BE DELIVERED TO THE PROJECT SITE AT A TEMPERATURE NOT LOWER THAN 250°F.
 - CONTRACTOR SHALL ESTABLISH AND MAINTAIN REQUIRED LINES AND ELEVATIONS FOR EACH COURSE DURING CONSTRUCTION.
 - BINDER COURSE AGGREGATE:
 - THE AGGREGATE FOR THE BINDER COURSE SHALL CONFORM TO SECTION 460 OF THE STATE HIGHWAY SPECIFICATIONS.
 - SURFACE COURSE AGGREGATE
 - THE AGGREGATE FOR THE SURFACE COURSE SHALL CONFORM TO SECTIONS 460 AND 465 OF THE STATE HIGHWAY SPECIFICATIONS.
 - ASPHALTIC MATERIALS SHALL CONFORM TO SECTIONS 455, 460, AND 465 OF THE STATE HIGHWAY SPECIFICATIONS.
- CONCRETE PAVING SPECIFICATIONS:**
 - CONCRETE PAVING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTIONS 405, 415, AND 416 OF THE STATE HIGHWAY SPECIFICATIONS.
 - CURING COMPOUNDS SHALL CONFORM TO SECTION 415 OF THE STATE HIGHWAY SPECIFICATIONS.
 - CONTRACTORS SHALL PROVIDE A JOINTING PLAN TO ENGINEER IF NOT INCLUDED IN THE PLANS. CONTRACTOR SHALL PROVIDE CONTROL JOINTS AND CONSTRUCTION JOINTS OF ONE-QUARTER CONCRETE THICKNESS AT AN EQUAL RATIO OF LENGTH TO WIDTH WHEREVER POSSIBLE WITH A MAXIMUM LENGTH BETWEEN JOINTS OF 15' ON CENTER.
 - CONTRACTOR SHALL PROVIDE EXPANSION JOINTS IN SIDEWALKS AT A MAXIMUM 100' APART.
 - PLACE EXPANSION JOINTS IN CURB, GUTTER, OR CURB AND GUTTER CONSTRUCTED NEXT TO ASPHALTIC PAVEMENT OR SURFACING. LOCATE JOINTS EVERYWHERE THAT TANGENT AND RADIAL CURB OR CURB AND GUTTER MEET; ON EACH SIDE OF EVERY INLET 3' FROM THE INLET, BUT NO CLOSER THAN 6" FROM ANOTHER JOINT; AND ON TANGENT SECTIONS PLACE BETWEEN 6' AND 300'.
 - IF CONSTRUCTING CURB, GUTTER, OR CURB AND GUTTER NEXT TO, OR ON, CONCRETE PAVEMENT CONSTRUCTED WITH EXPANSION JOINTS, THEN PLACE EXPANSION JOINTS TO MATCH THE EXPANSION JOINT LOCATIONS IN THE PAVEMENTS.
 - FOR CURB AND GUTTER, FORM CONTRACTION JOINTS BY SAWING OR FORMING AN INDUCED PLANE OF WEAKNESS AT LEAST 2" DEEP IN THE CURB, GUTTER, OR CURB AND GUTTER DIRECTLY OPPOSITE CONSTRUCTION OR CONTRACTION JOINTS IN ADJOINING CONCRETE PAVEMENT AND AT THE REQUIRED SPACING IN CURB, GUTTER, OR CURB AND GUTTER ADJOINING ASPHALTIC PAVEMENT. SPACE JOINTS BETWEEN 6' AND APPROXIMATELY 20' APART, AS THE ENGINEER DIRECTS.
 - EXTERIOR CONCRETE SURFACES SHALL BE BROOM FINISHED.
 - CONTRACTOR SHALL INSTALL TRUNCATED DOME WARNING DETECTION FIELD SHALL BE PLACED AT ALL ADA RAMPS AS SPECIFIED ON PLANS AND IN ACCORDANCE WITH STATE AND FEDERAL REGULABLES.
- PAVEMENT MARKING SPECIFICATIONS:**
 - ALL PARKING STALL LINES SHALL BE 4" WIDE, HIGH VISIBILITY YELLOW LATEX PAINT.
- PAVEMENT MARKINGS INCLUDING STOP BARS, CROSSWALKS, DIRECTIONAL ARROWS, PARKING STALL LINES, ADA STALL MARKINGS, NO PARKING ZONES, AND DROP-OFF/PICK-UP ZONES SHALL BE PAINTED WITH LATEX PAINT PER SPECIFICATIONS.

- CONCRETE PAVING SPECIFICATIONS:**
 - CONCRETE PAVING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF SECTIONS 405, 415, AND 416 OF THE STATE HIGHWAY SPECIFICATIONS.
 - CURING COMPOUNDS SHALL CONFORM TO SECTION 415 OF THE STATE HIGHWAY SPECIFICATIONS.
 - CONTRACTORS SHALL PROVIDE A JOINTING PLAN TO ENGINEER IF NOT INCLUDED IN THE PLANS. CONTRACTOR SHALL PROVIDE CONTROL JOINTS AND CONSTRUCTION JOINTS OF ONE-QUARTER CONCRETE THICKNESS AT AN EQUAL RATIO OF LENGTH TO WIDTH WHEREVER POSSIBLE WITH A MAXIMUM LENGTH BETWEEN JOINTS OF 15' ON CENTER.
 - CONTRACTOR SHALL PROVIDE EXPANSION JOINTS IN SIDEWALKS AT A MAXIMUM 100' APART.
 - PLACE EXPANSION JOINTS IN CURB, GUTTER, OR CURB AND GUTTER CONSTRUCTED NEXT TO ASPHALTIC PAVEMENT OR SURFACING. LOCATE JOINTS EVERYWHERE THAT TANGENT AND RADIAL CURB OR CURB AND GUTTER MEET; ON EACH SIDE OF EVERY INLET 3' FROM THE INLET, BUT NO CLOSER THAN 6" FROM ANOTHER JOINT; AND ON TANGENT SECTIONS PLACE BETWEEN 6' AND 300'.
 - IF CONSTRUCTING CURB, GUTTER, OR CURB AND GUTTER NEXT TO, OR ON, CONCRETE PAVEMENT CONSTRUCTED WITH EXPANSION JOINTS, THEN PLACE EXPANSION JOINTS TO MATCH THE EXPANSION JOINT LOCATIONS IN THE PAVEMENTS.
 - FOR CURB AND GUTTER, FORM CONTRACTION JOINTS BY SAWING OR FORMING AN INDUCED PLANE OF WEAKNESS AT LEAST 2" DEEP IN THE CURB, GUTTER, OR CURB AND GUTTER DIRECTLY OPPOSITE CONSTRUCTION OR CONTRACTION JOINTS IN ADJOINING CONCRETE PAVEMENT AND AT THE REQUIRED SPACING IN CURB, GUTTER, OR CURB AND GUTTER ADJOINING ASPHALTIC PAVEMENT. SPACE JOINTS BETWEEN 6' AND APPROXIMATELY 20' APART, AS THE ENGINEER DIRECTS.
 - EXTERIOR CONCRETE SURFACES SHALL BE BROOM FINISHED.
 - CONTRACTOR SHALL INSTALL TRUNCATED DOME WARNING DETECTION FIELD SHALL BE PLACED AT ALL ADA RAMPS AS SPECIFIED ON PLANS AND IN ACCORDANCE WITH STATE AND FEDERAL REGULABLES.
- PAVEMENT MARKING SPECIFICATIONS:**
 - ALL PARKING STALL LINES SHALL BE 4" WIDE, HIGH VISIBILITY YELLOW LATEX PAINT.
- PAVEMENT MARKINGS INCLUDING STOP BARS, CROSSWALKS, DIRECTIONAL ARROWS, PARKING STALL LINES, ADA STALL MARKINGS, NO PARKING ZONES, AND DROP-OFF/PICK-UP ZONES SHALL BE PAINTED WITH LATEX PAINT PER SPECIFICATIONS.

SEEDING AND RESTORATION NOTES

- CONTRACTOR SHALL PROVIDE NOTICE TO THE JURISDICTIONAL AUTHORITIES IN ADVANCE OF ANY SOIL DISTURBING ACTIVITIES, IN ACCORDANCE WITH MUNICIPAL REQUIREMENTS.
- ALL DISTURBED AREAS SHALL BE SODDED AND/OR SEEDED AND MULCHED IMMEDIATELY FOLLOWING GRADING ACTIVITIES. SOD/SEED MIX TO BE IN ACCORDANCE WITH LANDSCAPE PLAN.
- CONTRACTOR SHALL WATER ALL NEWLY SODDED/SEEDED AREAS DURING THE SUMMER MONTHS WHENEVER THERE IS A 7-DAY LAPSE WITH NO SIGNIFICANT RAINFALL.
- ALL SLOPES 20% OR GREATER SHALL BE TEMPORARILY SEEDED, MULCHED, OR OTHER MEANS OF COVER PLACED ON THEM WITHIN 2 WEEKS OF DISTURBANCE. REFER TO STABILIZATION PRACTICES IN THE EROSION CONTROL NOTES FOR FURTHER SPECIFICATIONS.
- CONTRACTOR SHALL CHISEL-PLOW OR DEEP TILL WITH DOUBLE TINES ALL STORMWATER MANAGEMENT FACILITIES JUST PRIOR TO SODDING AND/OR SEEDED AND MULCHING TO PROMOTE INFILTRATION.
- SEEDING SPECIFICATIONS:**
 - TURF LAWN SEED MIXTURE: WISDOT SEED MIX NO. 40 AT RATES SPECIFIED IN SECTION 630 OF THE STATE HIGHWAY SPECIFICATIONS
 - LOW MAINTENANCE AREA SEED MIXTURE: WISDOT SEED MIX NO. 10 OR 20 APPLIED AT RATES AS SPECIFIED IN SECTION 630 OF THE STATE HIGHWAY SPECIFICATIONS.
 - NO-MOW AREA SEED MIXTURE: NO-MOW LAWN SEED MIX, AS PROVIDED BY PRAIRIE NURSERY, P.O. BOX 306, WESTFIELD, WISCONSIN, 53984. TEL. 608-296-3679 (OR APPROVED EQUIVALENT). SEEDING RATE SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
- SEED PREPARATION SPECIFICATIONS:**
 - SCARIFY SUBSOILS TO A DEPTH OF 3" WHERE TOPSOIL SHALL BE PLACED TO REDUCE COMPACTION.
 - PLACE TOPSOIL AT A MINIMUM DEPTH OF 6" UNLESS OTHERWISE NOTED ON THE PLANS.
 - APPLY FERTILIZER IN ACCORDANCE WITH SEED MIX MANUFACTURER'S RECOMMENDATIONS
 - SOW SEED AT RATES SPECIFIED USING METHOD "A" OR METHOD "B" AS SPECIFIED IN SECTION 630 OF THE STATE HIGHWAY SPECIFICATIONS.
- SEED MULCHING/EROSION MATTING SPECIFICATIONS:**
 - ALL SEEDED AREAS WITH SLOPES FLATTER THAN 4:1, UNLESS OTHERWISE NOTED ON THE PLANS, SHALL BE STABILIZED WITH WEED-FREE WHEAT STRAW MULCH WITH METHODS AND RATES IN ACCORDANCE WITH SECTION 627 OF THE STATE HIGHWAY SPECIFICATIONS.
 - ALL SEEDED AREAS WITH SLOPES EQUAL TO OR STEEPER THAN 4:1, UNLESS OTHERWISE NOTED ON THE PLANS, SHALL BE STABILIZED WITH EROSION MATTING MATERIALS AS SPECIFIED ON THE PLANS. EROSION MATTING SHALL BE IN ACCORDANCE WITH SECTION 628 OF THE STATE HIGHWAY SPECIFICATIONS.

UTILITY NOTES

- ALL EXISTING UTILITIES ARE SHOWN FOR INFORMATIONAL PURPOSES ONLY AND ARE NOT GUARANTEED TO BE ACCURATE OR ALL INCLUSIVE. THE CONTRACTOR IS RESPONSIBLE FOR MAKING THEIR OWN DETERMINATION AS TO THE TYPE AND LOCATIONS OF UNDERGROUND UTILITIES AS MAY BE NECESSARY TO AVOID DAMAGE THERETO. CONTRACTOR/OWNER SHALL CALL "DIGGERS HOTLINE" PRIOR TO ANY CONSTRUCTION.
- PRIOR TO CONSTRUCTION, THE PRIME CONTRACTOR IS RESPONSIBLE FOR:
 - EXAMINING ALL SITE CONDITIONS RELATIVE TO THE CONDITIONS INDICATED ON THE ENGINEERING DRAWINGS. ANY DISCREPANCIES ARE TO BE REPORTED TO THE ENGINEER AND RESOLVED PRIOR TO THE START OF CONSTRUCTION.
 - OBTAINING ALL PERMITS INCLUDING PERMIT COSTS, TAP FEES, METER DEPOSITS, BONDS, AND ALL OTHER FEES REQUIRED FOR PROPOSED WORK TO OBTAIN OCCUPANCY.
 - VERIFYING ALL ELEVATIONS, LOCATIONS, AND SIZES OF SANITARY, WATER, AND STORM LATERALS AND CHECK ALL UTILITY CROSSINGS FOR CONFLICTS. NOTIFY ENGINEER OF ANY DISCREPANCY. NO WORK SHALL BE PERFORMED UNTIL THE DISCREPANCY IS RESOLVED.
 - NOTIFYING ALL UTILITIES PRIOR TO INSTALLATION OF ANY UNDERGROUND IMPROVEMENTS.
 - NOTIFYING THE DESIGN ENGINEER AND JURISDICTIONAL AUTHORITY 48 HOURS PRIOR TO THE START OF CONSTRUCTION TO ARRANGE FOR APPROPRIATE CONSTRUCTION OBSERVATION.
 - COORDINATING ALL CONSTRUCTION WITH OTHER CONTRACTORS INVOLVED WITH CONSTRUCTION OF THE PROPOSED DEVELOPMENT AND FOR REPORTING ANY ERRORS OR DISCREPANCIES BETWEEN THESE PLANS AND PLANS PREPARED BY OTHERS.
- ALL UTILITY WORK SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR PUBLIC UTILITIES AND STATE AND LOCAL CODES AND SPECIFICATIONS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE WHICH SPECIFICATIONS AND CODES APPLY, AND TO COORDINATE ALL CONSTRUCTION ACTIVITIES WITH THE APPROPRIATE JURISDICTIONAL AUTHORITIES.
- SPECIFICATIONS SHALL COMPLY WITH THE JURISDICTIONAL AUTHORITY'S SPECIAL PROVISIONS.
- LENGTHS OF ALL UTILITIES ARE TO CENTER OF STRUCTURES OR FITTINGS AND MAY VARY SLIGHTLY FROM PLAN. LENGTHS SHALL BE VERIFIED IN THE FIELD DURING CONSTRUCTION.
- CONTRACTOR SHALL BE RESPONSIBLE FOR SITE SAFETY DURING THE CONSTRUCTION OF IMPROVEMENTS.
- CONTRACTOR SHALL INSTALL A PEDESTRIAN FENCE AROUND ALL EXCAVATIONS TO BE LEFT OPEN OVERNIGHT AS REQUIRED IN CONSTRUCTION SITES WHERE THE POTENTIAL FOR PEDESTRIAN INJURY EXISTS.
- CONTRACTOR SHALL ADJUST AND/OR RECONSTRUCT ALL UTILITY COVERS (SUCH AS MANHOLE COVERS, VALVE BOX COVERS, ETC.) TO MATCH THE FINISHED GRADES OF THE AREAS EFFECTED BY THE CONSTRUCTION.
- ALL NON-METALLIC UTILITY PIPES (SANITARY SEWER, STORM SEWER, AND WATER PIPING) SHALL BE INSTALLED IN CONJUNCTION WITH TRACER WIRE AS REQUIRED BY SPS 382.30(1)(I)(H), SPS 382.36(7)(C)10, AND SPS 382.40(8)(K). COLOR OF TRACER WIRE SHALL BE: SANITARY SEWER - GREEN, STORM SEWER - BROWN, WATER - BLUE, NON-POTABLE WATER - PURPLE.
- DRY UTILITIES (COMMUNICATION, TELEPHONE, GAS, ELECTRIC, ETC.) ARE SHOWN FOR GENERAL ROUTING ONLY. CONTRACTOR SHALL COORDINATE DESIGN AND FINAL LOCATION WITH APPROPRIATE UTILITY COMPANY.
- THE PRIME CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL CONSTRUCTION WITH OTHER CONTRACTORS INVOLVED IN CONSTRUCTION OF THE PROPOSED DEVELOPMENT AND FOR REPORTING ANY ERRORS OR DISCREPANCIES BETWEEN THESE PLANS AND PLANS PREPARED BY OTHERS.
- ANY SANITARY SEWER, SANITARY SEWER SERVICES, WATER MAIN, WATER SERVICES, STORM SEWER, OR OTHER UTILITIES, WHICH ARE DAMAGED BY THE CONTRACTORS, SHALL BE REPAIRED TO THE JURISDICTIONAL AUTHORITY'S SATISFACTION AT THE CONTRACTOR'S EXPENSE.
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE ENGINEER WITH AS-BUILT CONDITIONS OF THE INSTALLED IMPROVEMENTS IN ORDER THAT THE APPROPRIATE DRAWINGS CAN BE PREPARED, IF REQUIRED. ANY CHANGES TO THE DRAWINGS OR ADDITIONAL ITEMS MUST BE REPORTED TO THE ENGINEER AS WORK PROGRESSES.
- IN ANY LOCATIONS WHERE BUILDING SEWERS (STORM AND SANITARY) ARE INSTALLED WITH LESS THAN THE MINIMUM COVER AS SPECIFIED IN SPS 382.30(1)(c) OR WATER PIPING 382.40(8)(a), CONTRACTOR SHALL INSTALL INSULATION IN ACCORDANCE WITH SPS 382.30(1)(c)2. FOR PROTECTION FROM FROST.
- STORM SEWER SPECIFICATIONS:**
 - PIPE:
 - REINFORCED CONCRETE PIPE (RCP) - SHALL MEET THE REQUIREMENTS OF ASTM CLASS III (MINIMUM) C76 WITH RUBBER GASKET JOINTS CONFORMING TO ASTM C443.
 - HIGH DENSITY DUAL-WALL POLYETHYLENE CORRUGATED PIPE (HDPE) - SHALL BE AS MANUFACTURED BY ADS OR EQUAL WITH WATER/TIGHT JOINTS, AND SHALL MEET THE REQUIREMENTS OF AASHTO DESIGNATION M294 TYPE "S".
 - POLYVINYL CHLORIDE (PVC) - SHALL MEET REQUIREMENTS OF ASTM D3034, SDR 35 FOR PIPE SIZES 8"-15" WITH INTEGRAL BELL TYPE FLEXIBLE ELECTROMETRIC JOINTS, MEETING THE REQUIREMENTS OF ASTM D3212, ASTM 1785 SCHEDULE 40 FOR PIPE DIAMETERS 4"-6". SDR 35 SHALL BE USED FOR DEPTHS 3'-15" AND SDR 26 FOR DEPTHS 16"-25" DEPENDENT ON LOCAL JURISDICTION.
 - INLETS AND CATCH BASINS:
 - INLETS SHALL BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 3.6.3.A AND DETAIL DRAWINGS FILE. NO. 28 OR 29 OF THE STANDARD SPECIFICATIONS, OR APPROVED EQUAL WITH A 2X3" MAXIMUM OPENING.
 - POLYVINYL CHLORIDE (PVC) INLETS BY NYLOPLAST ONLY WHEN SPECIFIED ON PLANS, CONFORMING TO ASTM D1781, ASTM D1782, ASTM F477, AND MANUFACTURER'S REQUIREMENTS. REFER TO PLANS FOR LID OR GRATE SPECIFICATION.
 - FRAME AND GRATE/LIDS:
 - CURB FRAME AND GRATES SHALL BE NEENAH R-3067 WITH TYPE "R" GRATE OR EQUAL, UNLESS AS SPECIFIED IN THE PLANS.
 - SOLID LID FRAME AND GRATES SHALL BE NEENAH R-1550, HEAVY DUTY NON-ROCKING SOLID LID OR EQUAL, UNLESS AS SPECIFIED IN THE PLANS.
 - GRATE FRAME AND GRATES SHALL BE NEENAH R-1550, HEAVY DUTY WITH A R-2578 GRATE OR EQUAL, UNLESS AS SPECIFIED IN THE PLANS.
 - MANHOLES:
 - MANHOLES SHALL BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 3.5.0 AND DETAIL DRAWINGS FILE NO. 11 AND/OR 12 OF THE STANDARD SPECIFICATIONS.
 - MANHOLE FRAMES AND COVERS SHALL BE NEENAH R-1550, HEAVY DUTY NON-ROCKING SOLID LID OR EQUAL, UNLESS AS SPECIFIED IN THE PLANS.
 - BACKFILL AND BEDDING:
 - STORM SEWER SHALL BE CONSTRUCTED WITH GRAVEL BACKFILL AND CLASS "B" BEDDING IN ALL PAVED AREAS AND TO A POINT 5' BEYOND THE EDGE OF PAVEMENT. TRENCHES RUNNING PARALLEL TO AND LESS THAN 5' FROM THE EDGE OF PAVEMENT SHALL ALSO REQUIRE GRAVEL BACKFILL.
 - LANDSCAPED AREAS MAY BE BACKFILLED WITH EXCAVATED MATERIAL IN CONFORMANCE WITH SECTION 8.4.3.5 OF THE STANDARD SPECIFICATIONS.
 - FIELD TILE CONNECTIONS:
 - ALL FIELD TILE ENCOUNTERED DURING CONSTRUCTION SHALL BE INCLUDED IN THE UNIT PRICE(S) FOR STORM SEWER. TILE LINES CROSSED BY THE TRENCH SHALL BE REPLACED WITH THE SAME MATERIAL AS THE STORM SEWER.
- WATER MAIN & WATER LATERAL SPECIFICATIONS:**
 - PIPE:
 - DUCTILE IRON PIPE SHALL BE CLASS 52 CONFORMING TO AWWA C151 AND CHAPTER 8.18.0 OF THE STANDARD SPECIFICATIONS.
 - POLYVINYL CHLORIDE PRESSURE PIPE (PVC) SHALL BE MANUFACTURED IN ACCORDANCE WITH AWWA C900 DR14 (CLASS 305) FOR SIZES UP TO 4" AND AWWA C900 DR18 (CLASS 235) UP TO 30" WITH INTEGRAL ELASTOMERIC BEAD AND GASKET JOINTS.
 - COPPER TYPE K TUBING SHALL CONFORM TO ASTM DESIGNATION 888 FOR WATER SERVICES LESS THAN 2" IN DIAMETER.
 - HIGH DENSITY POLYETHYLENE (HDPE) SHALL CONFORM TO THE REQUIREMENTS OF AWWA C901, SDR 9 MINIMUM FOR SIZES UP TO 3" AND TO AWWA C906, SDR 17 MINIMUM FOR SIZES GREATER THAN 3".
 - VALVES AND VALVE BOXES:
 - GATE VALVES SHALL BE AWWA GATE VALVES MEETING THE REQUIREMENTS OF AWWA C500 AND CHAPTER 8.27.0 OF THE STANDARD SPECIFICATIONS.
 - CURB STOPS AND CORPORATION VALVES SHALL BE AWWA C800 AND ASTM B62, AND CONFORM TO ANY LOCAL JURISDICTIONAL REQUIREMENTS.
 - WATER SERVICES CONNECTIONS:
 - SERVICES 2" IN DIAMETER OR LESS SHALL USE A TAP SERVICE WITH A CORPORATION STOP AND CURB STOP VALVE WITH SERVICE BOX PER JURISDICTIONAL REQUIREMENTS.
 - SERVICES GREATER THAN 2" IN DIAMETER SHALL USE A TAPPING VALVE OR CUT-IN TEE CONNECTION WITH VALVE OF EQUIVALENT PIPE DIAMETER AND VALVE BOX PER JURISDICTIONAL REQUIREMENTS.
 - HYDRANTS:
 - HYDRANTS SHALL CONFORM TO THE SPECIFICATIONS OF THE JURISDICTIONAL AUTHORITIES. THE DISTANCE FROM THE GROUND LINE TO THE CENTERLINE OF THE LOWEST NOZZLE AND THE LOWEST CONNECTION OF THE FIRE DEPARTMENT SHALL BE NO LESS THAN 18" AND NO GREATER THAN 23"(SEE DETAIL).
 - JOINT RESTRAINT:
 - WHERE SPECIFIED, DUCTILE IRON PIPE SHALL INCLUDE MECHANICAL JOINTS CONFORMING TO CHAPTER 4.4.2(b) OF THE STANDARD SPECIFICATIONS. POLYETHYLENE WRAP SHALL BE USED AROUND ALL MECHANICAL CONNECTIONS.
 - BEDDING AND COVER MATERIAL:
 - PIPE BEDDING AND COVER MATERIAL SHALL BE SAND, CRUSHED STONE CHIPS OR CRUSHED STONE SCREENINGS CONFORMING TO CHAPTER 8.4.3.5 OF THE STANDARD SPECIFICATIONS.
 - BURY DEPTH SHALL CONFORM TO LOCAL JURISDICTION REQUIREMENTS, OR DSPS REQUIREMENTS AT A MINIMUM, WHERE THERE IS NO LOCAL JURISDICTION REQUIREMENTS.
 - BACKFILL:
 - BACKFILL MATERIAL AND INSTALLATION SHALL BE IN ACCORDANCE WITH CHAPTERS 2.6.0 AND 4.17.0 OF THE STANDARD SPECIFICATIONS. GRAVEL BACKFILL IS REQUIRED IN ALL PAVED AREAS AND TO A POINT 5' BEYOND THE EDGE OF PAVEMENT. TRENCHES RUNNING PARALLEL TO AND LESS THAN 5' FROM THE EDGE OF PAVEMENT SHALL ALSO REQUIRE GRAVEL BACKFILL.
 - LANDSCAPED AREAS MAY BE BACKFILLED WITH EXCAVATED MATERIAL IN CONFORMANCE WITH SECTION 8.4.3.5 OF THE STANDARD SPECIFICATIONS.
- SEPARATION DISTANCES
 - WHERE PRIVATE WATER MAIN OR WATER SERVICES CROSSES A SANITARY SEWER OR SANITARY LATERAL, THE WATER PIPE WITHIN 5 FEET OF THE CROSSING SHALL BE INSTALLED WITH THE FOLLOWING:
 - WATER PIPING SHALL BE INSTALLED AT LEAST 12 INCHES ABOVE THE TOP OF SANITARY PIPING.
 - WATER PIPING SHALL BE INSTALLED AT LEAST 18 INCHES BELOW THE BOTTOM OF SANITARY PIPING.

UTILITY NOTES, CONTINUED

- SANITARY SEWER SPECIFICATIONS:**
 - PIPE:
 - POLYVINYL CHLORIDE (PVC) MEETING REQUIREMENTS OF ASTM D 3034, WITH INTEGRAL BELL TYPE FLEXIBLE ELASTOMERIC JOINTS, MEETING THE REQUIREMENTS OF ASTM D3212, ASTM 1785 SCHEDULE 40 FOR PIPE DIAMETERS 4"-6". SDR 35 SHALL BE USED FOR DEPTHS 3'-15" AND SDR 26 FOR DEPTHS 16"-25" DEPENDENT ON LOCAL JURISDICTION.
 - CONNECTION TO DISSIMILAR PIPE MATERIALS SHALL CONFORM TO CHAPTER 3.4.2 OF THE STANDARD SPECIFICATIONS. FERNOCO COUPLER MAY BE USED WITH APPROVAL OF ENGINEER.
 - MANHOLES:
 - MANHOLES SHALL BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 3.5.0 AND DETAIL DRAWINGS FILE NOS. 12, 13 AND 15 OF THE STANDARD SPECIFICATIONS AND ALL SPECIAL PROVISIONS OF THE JURISDICTIONAL AUTHORITIES.
 - MANHOLES SHALL HAVE INTERNAL CHIMNEY SEALS INSTALLED IN ALL SANITARY MANHOLES IN ACCORDANCE WITH CHAPTER 3.5.4(F) AND DETAIL DRAWING FILE NO. 12A OF THE STANDARD SPECIFICATIONS.
 - MANHOLES SHALL HAVE ALL EXTERNAL JOINTS WRAPPED WITH MAC WARP OR EQUAL RUBBERIZED JOINT WRAP PER ASTM C923.
 - MANHOLE FRAMES AND COVERS SHALL BE NEENAH R-1550 HEAVY DUTY WITH NON-ROCKING SOLID LIDS OR EQUAL, UNLESS SPECIFIED IN THE PLANS.
 - BEDDING AND COVER MATERIAL:
 - MATERIAL SHALL CONFORM TO THE APPROPRIATE SECTIONS OF THE STANDARD SPECIFICATIONS WITH THE FOLLOWING MODIFICATION: "COVER MATERIAL SHALL BE THE SAME AS USED FOR BEDDING AND SHALL ACCORDANCE WITH LOCAL CODES AND SPECIFICATIONS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE WHICH SPECIFICATIONS AND CODES APPLY, AND TO COORDINATE ALL CONSTRUCTION ACTIVITIES WITH THE APPROPRIATE JURISDICTIONAL AUTHORITIES.
 - MATERIAL SHALL BE PLACED IN A MINIMUM OF THREE SEPARATE LIFTS, OR AS REQUIRED TO ENSURE ADEQUATE COMPACTION OF THESE MATERIALS, WITH ONE LIFT OF BEDDING MATERIAL ENDING AT OR NEAR THE SPRINGLINE OF THE PIPE. THE CONTRACTOR SHALL TAKE CARE TO COMPLETELY WORK BEDDING MATERIAL UNDER THE HAUNCH OF THE PIPE TO PROVIDE ADEQUATE SIDE SUPPORT."
 - BACKFILL:
 - MATERIAL AND INSTALLATION SHALL BE IN ACCORDANCE CHAPTER 2.6.0 OF THE STANDARD SPECIFICATIONS. GRAVEL BACKFILL IS REQUIRED IN ALL PAVED AREAS AND TO A POINT 5' BEYOND THE EDGE OF PAVEMENT. TRENCHES RUNNING PARALLEL TO AND LESS THAN 5' FROM THE EDGE OF PAVEMENT SHALL ALSO REQUIRE GRAVEL BACKFILL.
 - LANDSCAPED AREAS MAY BE BACKFILLED WITH EXCAVATED MATERIAL IN CONFORMANCE WITH SECTION 8.4.3.5 OF THE STANDARD SPECIFICATIONS.

EROSION CONTROL NOTES

- CONTRACTOR IS RESPONSIBLE TO NOTIFY ENGINEER OF RECORD AND OFFICIALS OF ANY CHANGES TO THE EROSION CONTROL AND STORMWATER MANAGEMENT PLANS.
- ALL EROSION CONTROL MEASURES SHALL BE CONSTRUCTED AND MAINTAINED BY THE CONTRACTOR IN ACCORDANCE WITH JURISDICTIONAL REQUIREMENTS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN A COPY OF THESE STANDARDS. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL EROSION CONTROL MEASURES WHICH MAY BE NECESSARY TO MEET UNFORESEEN FIELD CONDITIONS.
- INSTALL PERIMETER EROSION CONTROL MEASURES (SUCH AS CONSTRUCTION ENTRANCES, SILT FENCE, AND EXISTING INLET PROTECTION) PRIOR TO ANY GRADING OR DISTURBANCE OF EXISTING SURFACE COVER. MODIFICATIONS TO THE APPROVED EROSION CONTROL DESIGN IN ORDER TO MEET UNFORESEEN FIELD CONDITIONS IS ALLOWED IF MODIFICATIONS CONFORM TO WDNR TECHNICAL STANDARDS AND JURISDICTIONAL REQUIREMENTS. ALL DESIGN MODIFICATIONS MUST BE APPROVED BY THE JURISDICTIONAL AUTHORITIES PRIOR TO DEVIATION OF THE APPROVED PLAN.
- ADDITIONAL EROSION CONTROL MEASURES, AS REQUESTED BY JURISDICTIONS HAVING AUTHORITY AND/OR ENGINEER OF RECORD SHALL BE INSTALLED WITHIN 24 HOURS OF REQUEST.
- INSPECTIONS AND MAINTENANCE OF ALL EROSION CONTROL MEASURES SHALL BE ROUTINE (ONCE PER WEEK MINIMUM) TO ENSURE PROPER FUNCTION OF EROSION CONTROLS AT ALL TIMES. EROSION CONTROL MEASURES ARE TO BE IN WORKING ORDER AT THE END OF EACH WORK DAY.
- ALL EROSION AND SEDIMENT CONTROL ITEMS SHALL BE INSPECTED WITHIN 24 HOURS OF ALL RAIN EVENTS EXCEEDING 0.5". ANY DAMAGED EROSION CONTROL MEASURES SHALL BE REPAIRED OR REPLACED IMMEDIATELY UPON INSPECTION.
- CONSTRUCTION ENTRANCES SHALL BE INSTALLED AT ALL LOCATIONS OF VEHICLE INGRESS/EGRESS POINTS. ADDITIONAL LOCATIONS OTHER THAN AS SHOWN ON THE PLANS MUST BE PRE-APPROVED BY THE JURISDICTION. CONSTRUCTION ENTRANCES SHALL BE 50' LONG AND NO LESS THAN 12" THICK BY USE OF 3" SELECTED CRUSHED CONSTRUCTION SAND. CONSTRUCTION ENTRANCES SHALL BE INSTALLED BY THE CONTRACTOR IN A CONDITION WHICH WILL PREVENT THE TRACKING OF MUD OR DRY SEDIMENT OFF-SITE AFTER EACH WORKING DAY OR MORE FREQUENTLY AS REQUIRED.
- PAVED SURFACES ADJACENT TO CONSTRUCTION SITE VEHICLE ACCESS SHALL BE SWEEP AND/OR SCRAPED TO REMOVE ACCUMULATED SOIL, DIRT, AND/OR DUST AFTER THE END OF EACH WORK DAY AND AS REQUESTED BY THE JURISDICTIONAL AUTHORITIES.
- INLET PROTECTION SHALL BE IMMEDIATELY FITTED AT THE INLETS OF ALL INSTALLED STORM SEWER, STONE DITCH CHECKS, FENCE SHALL BE IMMEDIATELY FITTED AT ALL INSTALLED CULVERT INLETS TO PREVENT SEDIMENT DEPOSITION WITHIN STORM SEWER SYSTEMS.
- INSTALL EROSION CONTROLS ON THE DOWNSTREAM SIDE OF STOCKPILES. IF STOCKPILE REMAINS UNDISTURBED FOR MORE THAN SEVEN (7) DAYS, TEMPORARY SEEDING AND STABILIZATION IN ACCORDANCE WITH BEST MANAGEMENT PRACTICES IS REQUIRED. IF DISTURBANCE OCCURS BETWEEN FIFTEEN (15) AND MAY 15TH, THE MULCHING SHALL BE PERFORMED BY HYDRO-MULCHING WITH A "TACKIFIER."
- DITCH CHECKS AND APPLICABLE EROSION NETTING/MATTING SHALL BE INSTALLED IMMEDIATELY AFTER COMPLETION OF GRADING EFFORTS WITHIN DITCHES/SWALES TO PREVENT SOIL TRANSPORTATION.
- EROSION CONTROL FOR UTILITY CONSTRUCTION (STORM SEWER, SANITARY SEWER, WATER MAIN, ETC.):
 - PLACE EXCAVATED TRENCH MATERIAL ON THE HIGH SIDE OF THE TRENCH.
 - BACKFILL, COMPACT, AND STABILIZE THE TRENCH IMMEDIATELY AFTER PIPE CONSTRUCTION.
 - DISCHARGE TRENCH WATER INTO A SEDIMENTATION BASIN OR FILTERING TANK IN ACCORDANCE WITH THE WDNR DEWATERING TECHNICAL STANDARD NO. 1061 PRIOR TO RELEASE INTO THE STORM SEWER, RECEIVING STREAM, OR DRAINAGE DITCH.
- ALL SLOPES 4:1 OR GREATER SHALL BE STABILIZED WITH CLASS I, TYPE B EROSION MATTING PER STATE HIGHWAY SPECIFICATIONS OR APPLICATION OF A WISDOT APPROVED POLYMER SOIL STABILIZATION TREATMENT OR A COMBINATION THEREOF, AS REQUIRED WITHIN SEVEN (7) DAYS OF REACHING FINAL GRADE. DRAINAGE SWALES SHALL BE STABILIZED WITH CLASS II, TYPE B EROSION MATTINGS PER STATE

GRADING AND EARTHWORK NOTES

- ALL SITE PREP AND EARTHWORK SHALL CONFORM TO THE GEOTECHNICAL REPORT PREPARED BY INTERTEK PSI. ISSUE DATE 01/15/24. ALL REFERENCES TO THE "GEOTECHNICAL REPORT" SHALL BE UNDERSTOOD AS THE AFOREMENTIONED REPORT.
- ALL PROPOSED GRADES SHOWN ARE FINISHED GRADES. CONTRACTOR SHALL VERIFY ALL GRADES, MAKE SURE ALL AREAS DRAIN PROPERLY, AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.
- CONTRACTOR SHALL ASSUME SOLE RESPONSIBILITY FOR COMPUTATIONS OF ALL GRADING QUANTITIES. WHILE JSD PROFESSIONAL SERVICES, INC. ATTEMPTS TO PROVIDE A COST-EFFECTIVE APPROACH TO BALANCE EARTHWORK, GRADING DESIGN IS BASED ON MANY FACTORS, INCLUDING SAFETY, AESTHETICS, AND COMMON ENGINEERING STANDARDS OF CARE. THEREFORE, NO GUARANTEE CAN BE MADE FOR A BALANCED SITE.
- ALL EXCAVATIONS AND FILLS SHALL BE TO THE ELEVATIONS SHOWN ON THE DRAWINGS AND SHALL INCLUDE SUFFICIENT DEPTHS FOR PLACEMENT OF FILL MATERIALS, BASE COURSES, PAVEMENTS, TOPSOIL, AND OTHER MATERIALS TO THE SPECIFIED DEPTHS.
- CONTRACTOR SHALL NOT EXCAVATE BELOW ELEVATIONS OR DESIGN GRADES SHOWN ON THE DRAWINGS WITHOUT PRIOR AUTHORIZATION FROM ENGINEER AND OWNER.
- PRIOR TO ALL EXCAVATION OR FILLING OPERATIONS, CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL TOPSOIL FROM PROPOSED LOCATIONS OF BUILDINGS, STRUCTURES, ROADS, WALKS, OTHER PAVED AREAS, STORM WATER FACILITIES OR WITHIN THE GRADING EXTENTS WHERE EXISTING GRADES ARE ALTERED BY MORE THAN 3". REMOVED OR STRIPPED TOPSOIL SHALL BE SEGREGATED AND STOCKPILED ON-SITE IN AN APPROPRIATE LOCATION TO BE RESPREAD AS SPECIFIED ON THE DRAWINGS.
- CONTRACTOR SHALL NOT PLACE ANY FILL OR OTHER MATERIALS ON AREAS THAT HAVE NOT HAD TOPSOIL REMOVED, ARE FROZEN, SATURATED, OR YIELDING. CONTRACTOR SHALL NOTIFY OWNER OR ENGINEER IF SUBGRADE CONDITIONS ARE NOT SUITABLE FOR SUPPORTING FILL AND A FURTHER DETERMINATION SHALL BE PROVIDED BY OWNER OR ENGINEER.
- CONTRACTOR SHALL PLACE THE FILLS IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT INCLUDING LIFT DEPTHS AND COMPACTION EFFORTS.
- PRIOR TO PLACEMENT OF BASE COURSE MATERIALS IN PAVEMENT OR HARD SURFACE AREAS OR CONDUCTING EXCAVATION BELOW SUBGRADE (EBS) ELEVATIONS, CONTRACTOR SHALL NOTIFY OWNER AND ENGINEER TO CONDUCT AN INSPECTION OF THE PREPARED SUBGRADE AND PROOF-ROLLING. PROOF-ROLLING SHALL BE CONDUCTED BY THE CONTRACTOR IN WITNESS OF THE OWNER AND ENGINEER. OWNER AND ENGINEER SHALL DETERMINE IF AREAS OF EBS ARE REQUIRED. EBS SHALL BE COMPLETED BY THE CONTRACTOR PER THE DIRECTION OF THE OWNER AND ENGINEER.
- SOIL MATERIAL SPECIFICATIONS:
 - FILL AND BACKFILL MATERIALS
 - MATERIAL SHALL BE SATISFACTORY MATERIALS EXCAVATED FROM THE SITE, PER THE GEOTECHNICAL REPORT. IF SATISFACTORY MATERIALS ARE NOT AVAILABLE ONSITE OR ADDITIONAL MATERIALS ARE REQUIRED, REFER TO IMPORTED FILL MATERIAL SPECIFICATIONS.
 - IMPORTED FILL MATERIAL
 - MATERIAL SHALL BE PROVIDED BY THE CONTRACTOR FROM OFFSITE BORROW AREAS WHEN SUFFICIENT, SATISFACTORY MATERIALS ARE NOT AVAILABLE ONSITE. IMPORTED FILL MATERIAL SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL REPORT AND CONSIST OF CLEAN MATERIAL OF INORGANIC SOILS OR A MIXTURE OF INORGANIC SOIL AND ROCK, STONE, OR GRAVEL. THE MATERIAL SHALL BE FREE OF TOPSOIL, VEGETATION, PAVEMENT RUBBLE, DEBRIS, OR OTHER DELETERIOUS MATERIALS. THE MAXIMUM NOMINAL DIMENSION OF MATERIALS CONSISTING OF ROCK, STONE, OR GRAVEL SHALL BE 6".
 - GRANULAR FILL
 - MATERIAL SHALL CONSIST OF CLEAN MATERIAL MEETING THE REQUIREMENTS OF "GRADE 1" OR "GRADE 2" GRANULAR BACKFILL AS DEFINED IN SECTION 209.2.1 OF THE STATE HIGHWAY SPECIFICATIONS.

CONSTRUCTION SITE DEWATERING NOTES

- ALL CONSTRUCTION SITE DEWATERING PRACTICES SHALL BE IN ACCORDANCE WITH WDNr TECHNICAL STANDARD 1061 FOR PUMPING DISCHARGE RATES OF LESS THAN 70 GPM. CONTRACTOR SHALL BE RESPONSIBLE FOR APPLICABLE JURISDICTIONAL DEWATERING PERMITS. FOR DISCHARGE RATES EXCEEDING 70 GPM, CONTRACTOR SHALL BE RESPONSIBLE FOR PERMITTING OF DEWATERING SYSTEM AS A HIGH CAPACITY DEWATERING WELL WITH WDNr AND LOCAL JURISDICTIONS.
- CONTRACTOR SHALL DISCHARGE TREATED EFFLUENT FROM DEWATERING SYSTEM TO LOCATIONS IN ACCORDANCE WITH WDNr TECHNICAL STANDARD 1061. CONTRACTOR SHALL OBTAIN APPROVAL FROM LOCAL JURISDICTION OR ADJACENT PRIVATE PROPERTY OWNERS FOR ALL DISCHARGES TO JURISDICTION'S PUBLIC INFRASTRUCTURE OR PRIVATE PROPERTY.
- IN THE EVENT OF DIRTY/UNCLEAR DISCHARGE, CONTRACTOR SHALL HALT PUMPING OPERATIONS AND EVALUATE THE DEWATERING TREATMENT/FILTERING SYSTEM.
- IN THE EVENT CONTRACTOR FINDS SIGNS OF CONTAMINATION IN THE GROUNDWATER OR SITE SOILS, ALL DEWATERING PRACTICES SHALL BE HALTED AND CONTAMINATION SHALL BE EVALUATED BY A QUALIFIED PROFESSIONAL TO PREVENT TRANSPORTATION OF CONTAMINATES DOWNSTREAM OF DISCHARGES.
 - IF DEWATERING PRACTICES CONTINUE WITH CONTAMINATED GROUNDWATER OR SOIL CONDITIONS, THE CONTRACTOR SHALL BE RESPONSIBLE TO OBTAIN ALL REQUIRED PERMITS AND APPROVALS TO CONTINUE OPERATION OF DEWATERING SYSTEMS.
 - UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR DISCHARGE CONSTRUCTION SITE DEWATERING PRACTICES TO THE SANITARY SEWER WITHOUT AUTHORIZATION AND PERMIT APPROVALS FROM THE LOCAL JURISDICTION.

STORMWATER FACILITIES CONSTRUCTION NOTES

- ENGINEER SHALL BE NOTIFIED PRIOR TO INSTALLATION OF STORMWATER MANAGEMENT FACILITIES. CONTRACTOR SHALL PROVIDE ENGINEER DOCUMENTATION OF CONSTRUCTION OF STORMWATER FACILITIES INCLUDING GEOTECHNICAL NOTES ON EXISTING SOILS, PHOTOGRAPHS OF EACH STAGE OF BASIN CONSTRUCTION, LOAD TICKETS AND SPECIFICATION OF MATERIALS USED IN THE BASIN, AND ELEVATION OF CRITICAL ITEMS SUCH AS PIPES, EXCAVATIONS, DRAINAGE STONE BOTTOM, ETC.
- ERECT CONSTRUCTION FENCING OR SNOW FENCING AROUND PROPOSED STORMWATER FACILITIES INTENDED TO INFILTRATE STORMWATER TO PROTECT THESE SOILS FROM DISTURBANCE AND COMPACTION. CONSTRUCTION TRAFFIC, HEAVY EQUIPMENT, AND SOIL STOCKPILES SHALL NOT BE PLACED IN AREAS WHERE PROPOSED INFILTRATE AREAS ARE LOCATED.
- STORMWATER MANAGEMENT FACILITIES FOR INFILTRATION SHALL BE INSTALLED AFTER SUBSTANTIAL COMPLETION OF FINAL SITE GRADING AND SOILS HAVE BEEN STABILIZED.
- TEMPORARY SEDIMENT BASINS SHALL BE REMOVED IN THEIR ENTIRETY AFTER CONSTRUCTION OF STORMWATER MANAGEMENT FACILITIES. PERMANENT STORMWATER BASINS USED AS TEMPORARY SEDIMENT BASINS SHALL BE DREDGED TO DESIGN DEPTH AFTER SITE IS STABILIZED.
- REFER TO PROJECT DETAILS FOR FURTHER INFORMATION AND SPECIFICATIONS FOR THE CONSTRUCTION OF THE STORMWATER FACILITIES.

LEGEND

- PROPERTY LINE
- RIGHT-OF-WAY
- EASEMENT LINE
- BUILDING SETBACK LINE
- EDGE OF PAVEMENT
- STANDARD CURB AND GUTTER
- ASPHALT PAVEMENT
- CONCRETE PAVEMENT
- PROPOSED 1 FOOT CONTOUR
- PROPOSED 5 FOOT CONTOUR
- EXISTING 1 FOOT CONTOUR
- EXISTING 5 FOOT CONTOUR
- DRAINAGE DIRECTION
- GRADE BREAK
- STORMWATER MANAGEMENT AREA
- SANITARY SEWER
- WATERMAIN
- STORM SEWER
- EXISTING SANITARY SEWER
- EXISTING WATERMAIN
- EXISTING STORM SEWER
- SILT FENCE
- RIP-RAP
- CONSTRUCTION ENTRANCE
- EROSION MATTING
- TURF REINFORCEMENT MATTING
- DITCH CHECK
- INLET PROTECTION
- DEMOLITION - REMOVAL OF ONSITE CURB SURFACES AND BASE COURSE
- SAWCUT EXISTING PAVEMENT
- DEMOLITION - REMOVAL OF ASPHALT SURFACES
- DEMOLITION - REMOVAL OF CONCRETE SURFACES
- DEMOLITION - REMOVAL OF UTILITIES
- DEMOLITION - REMOVAL OF LANDSCAPE
- DEMOLITION - CLEAR AND GRUB BOUNDARY



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CLIENT:
THOMPSON ENGINEERING LLC

CLIENT ADDRESS:
**918 DEMING WAY, FLOOR 3
MADISON, WI 53717**

PROJECT:
LOT 115 OAK MEADOWS

PROJECT LOCATION:
**2621 OAK MEADOWS DRIVE
FITCHBURG, DANE COUNTY
WI, 53711**

PLAN MODIFICATIONS:

#	Date:	Description:
1	05.10.24	P-PLAT SUBMITTAL
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Designed By: CAC/CHG
Reviewed By: CWJ
Approved By: MRH

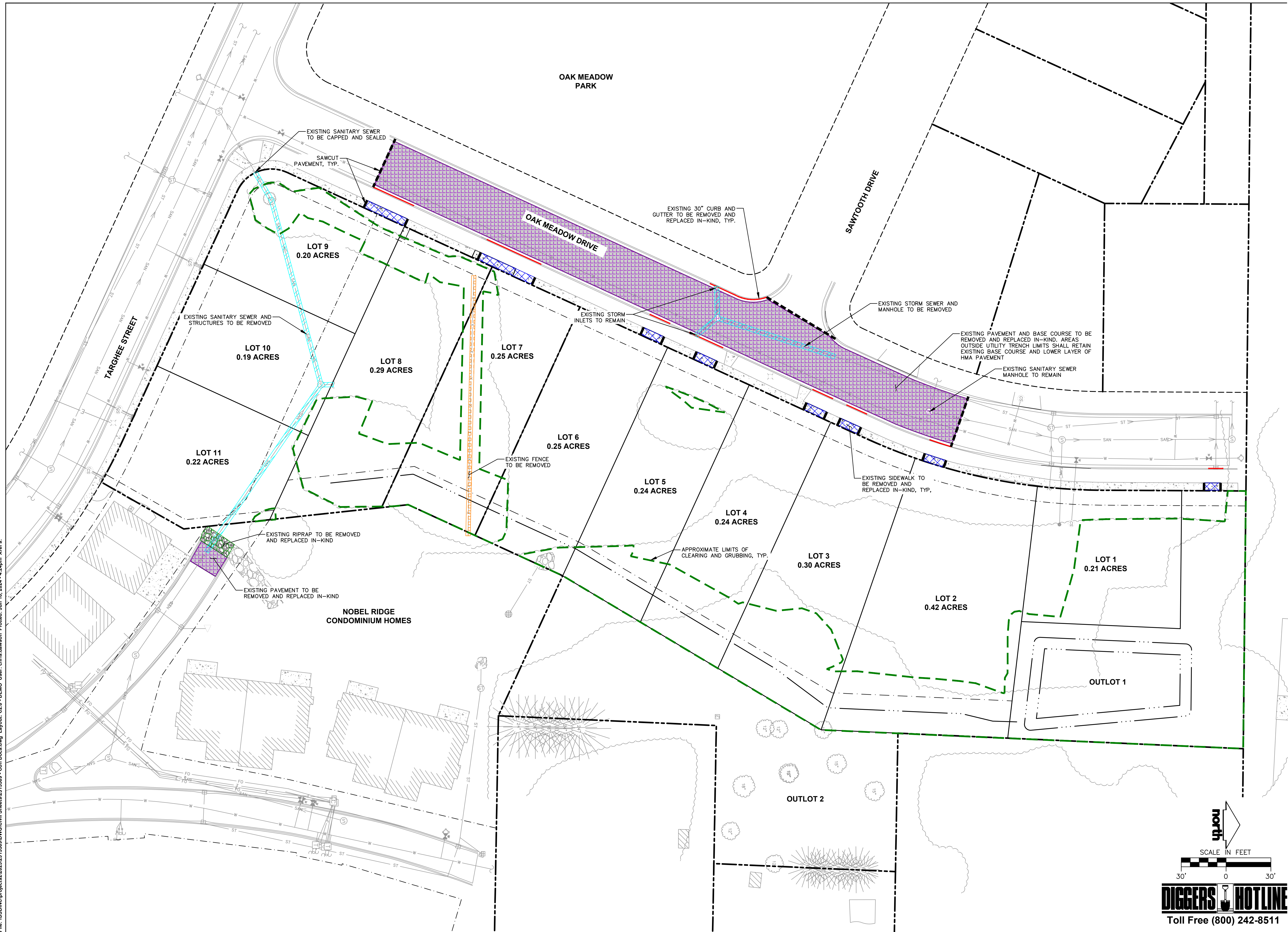
NOTES & LEGEND

SHEET NUMBER:

C1.1

JSD PROJECT NO: 23-13509

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PROJECT:
LOT 115 OAK MEADOWS

PROJECT LOCATION:
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FITZBURG, DANE COUNTY
WI, 53711**

PLAN MODIFICATIONS:

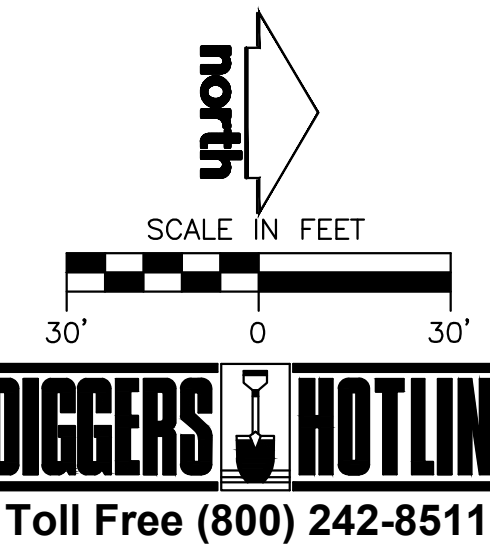
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Designed By: CAC/CHG
Reviewed By: CWD
Approved By: MRH

SHEET TITLE:
DEMOLITION PLAN

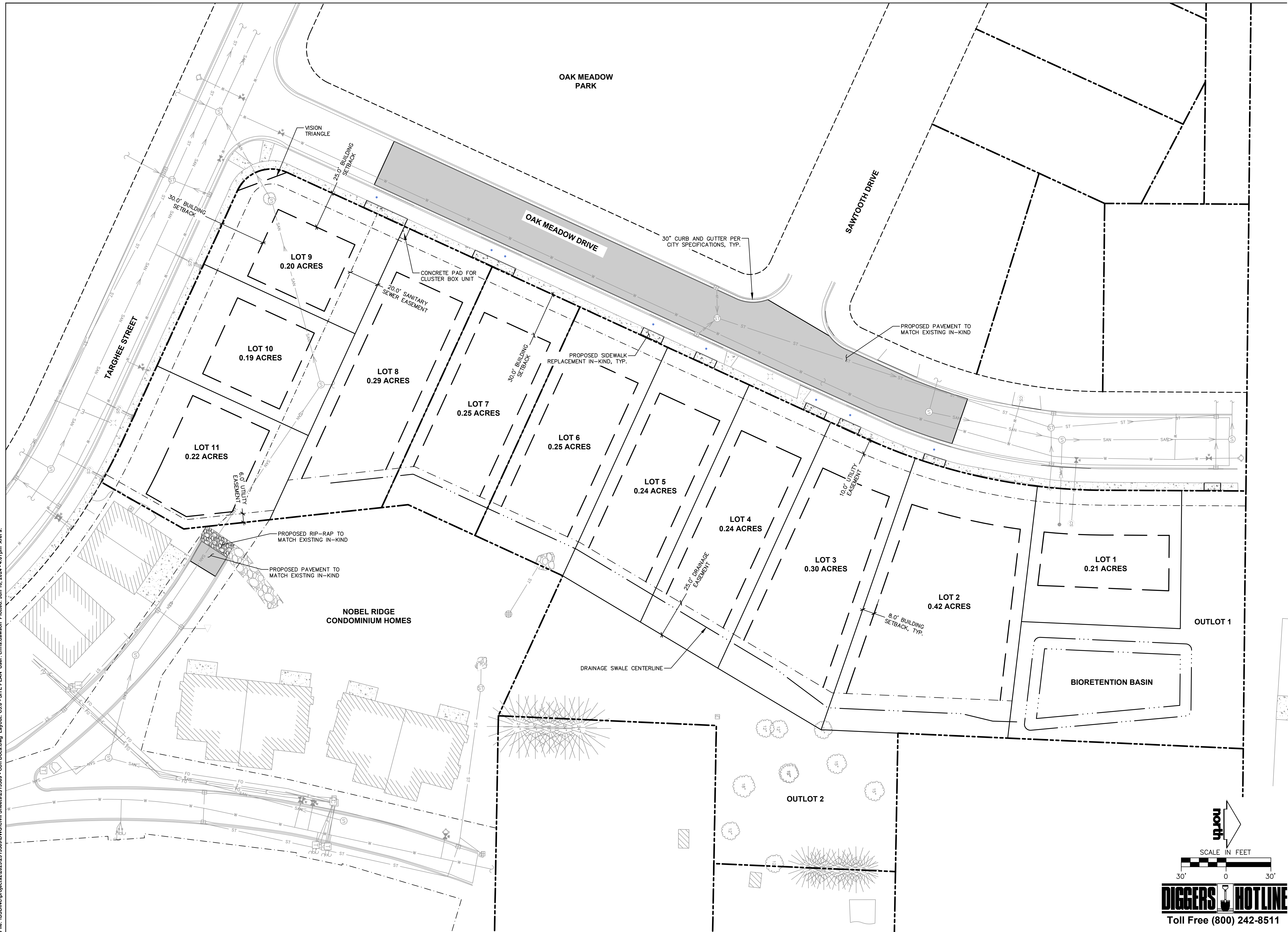
SHEET NUMBER:
C2.0

JSD PROJECT NO: 23-13509



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PROJECT:
LOT 115 OAK MEADOWS

PROJECT LOCATION:
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FITCHBURG, DANE COUNTY
WI, 53711**

PLAN MODIFICATIONS:

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Designed By: CAC/CHG
Reviewed By: CWD
Approved By: MRH

SHEET TITLE:
SITE PLAN

SHEET NUMBER:
C3.0

JSD PROJECT NO: 23-13509

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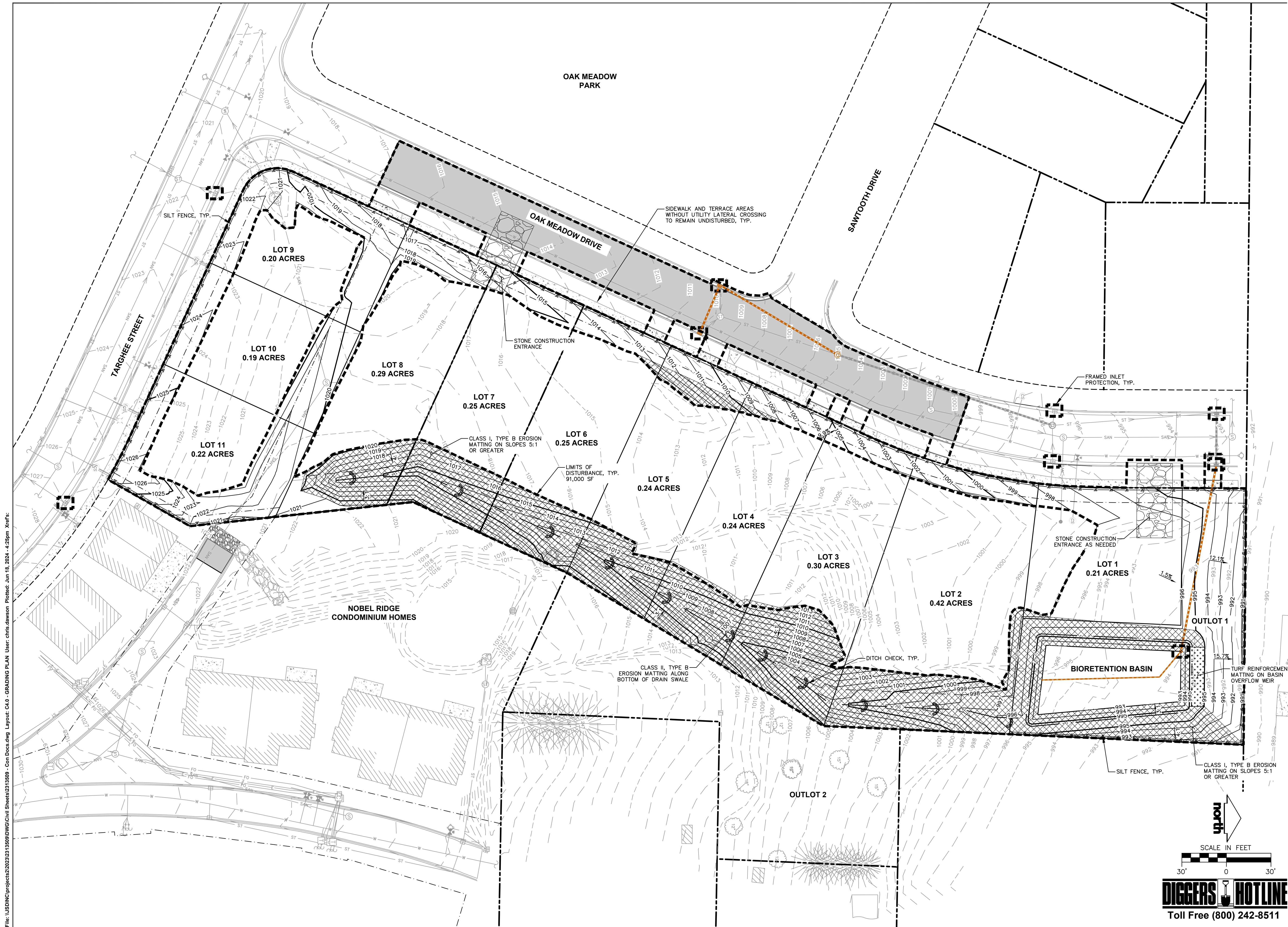
Designed By: CAC/CHG
Reviewed By: CWB
Approved By: MRH

**GRADING & EROSION
CONTROL PLAN**

SHEET NUMBER:

C4.0

JSD PROJECT NO: 23-13509



File: JSD\INC\projects\2023\2313509\DWG\Civil\Sheets\C4.0 - GRADING PLAN User: chris.dawson Plotted: Jun 18, 2024 - 4:25pm Xref's:

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Designed By: CAC/CHG
Reviewed By: CWJ
Approved By: MRH

SHEET TITLE:
UTILITY PLAN

SHEET NUMBER:

C5.0

JSD PROJECT NO: 23-13509



LABEL	RIM EL. (FT)	INVERT EL. (FT)	DEPTH (FT)	STRUCTURE DESC.	FRAME & GRATE
EX STO INL-10	992.84	W INV: 989.59 (12") E INV: 989.69 (12")	3.3	2 x 3 INLET	R-3067 TYPE L
STO CB B-1	995.95	W INV: 990.50 (12") SE INV: 990.50 (6")	5.5	48 IN MH (FLAT)	48" HAALA TRASH RACK
STO INL A-2	1009.79	NE INV: 1005.44 (12") SE INV: 1005.54 (12")	4.3	2 x 3 INLET	R-3067 TYPE L
STO INL A-3	1009.73	NW INV: 1006.25 (12")	3.5	2 x 3 INLET	R-3067 TYPE L
STO MH A-1	1005.03	N INV: 1000.19 (12") SW INV: 1000.29 (12")	4.8	48 IN MH (FLAT)	R-1550 SOLID LID

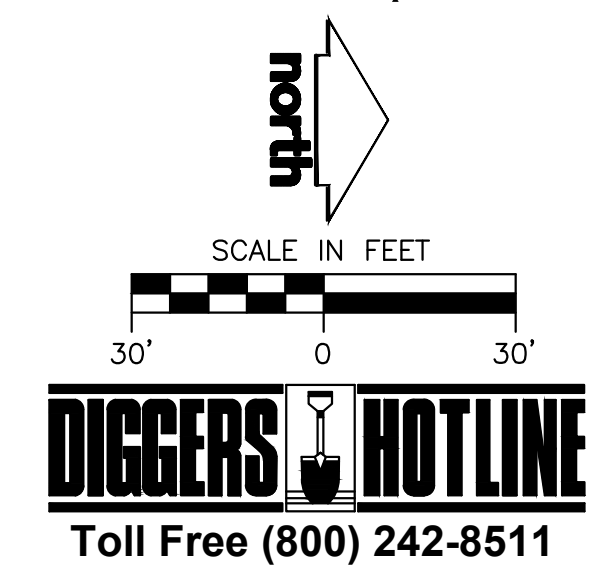
LABEL	RIM EL. (FT)	INVERT EL. (FT)	DEPTH (FT)	STRUCTURE DESC.	FRAME & GRATE
EX SAN MH-6	1000.79	N INV: 988.16 (12") SW INV: 988.49 (8")	12.6	48 IN MH	R-1550 SOLID LID
SAN MH A-1	1016.53	NE INV: 1003.73 (8") SE INV: 1003.83 (8")	12.9	48 IN MH	R-1550 SOLID LID
SAN MH A-2	1021.62	NW INV: 1007.51 (8") SE INV: 1007.61 (8")	14.1	48 IN MH	R-1550 SOLID LID
SAN MH A-3	1021.50	NW INV: 1008.41 (8") SE INV: 1010.20 (8")	13.2	48 IN MH	R-1550 SOLID LID

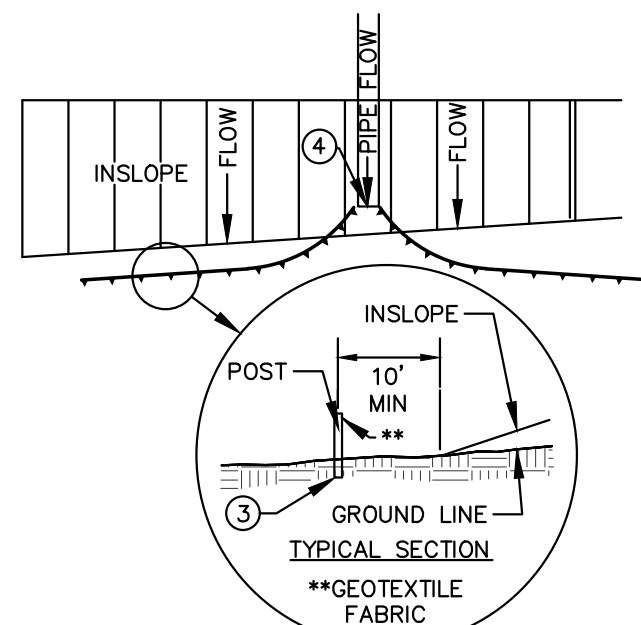
LABEL	FROM	TO	LENGTH	INVERT EL. (FT)	DISCHARGE EL. (FT)	SLOPE	SIZE & MATERIAL
STO A-2	STO INL A-2	STO MH A-1	92'	1005.44	1000.29	5.60%	12 IN RCP
STO A-3	STO INL A-3	STO INL A-2	36'	1006.25	1005.54	2.00%	12 IN RCP
STO B-1	STO CB B-1	EX STO INL-10	127'	990.50	989.69	0.64%	12 IN RCP
STO B-2	STO B-2	STO CB B-1	23'	990.50	990.50	0.00%	6 IN HDPE
STO B-3	STO B-3	STO B-2	79'	990.50	990.50	0.00%	6 IN HDPE

LABEL	FROM	TO	LENGTH	INVERT EL. (FT)	DISCHARGE EL. (FT)	SLOPE	SIZE & MATERIAL
SAN A-1	SAN MH A-1	EX SAN MH-6	381'	1003.73	988.49	4.00%	8 IN PVC
SAN A-2	SAN MH A-2	SAN MH A-1	245'	1007.51	1003.83	1.50%	8 IN PVC
SAN A-3	SAN MH A-3	SAN MH A-2	53'	1008.41	1007.61	1.50%	8 IN PVC

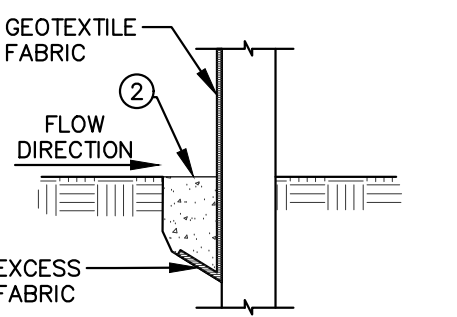
File: \\JSD\proj\2023\2313509\DWG\Civil\Sheets\C5.0 - Utility Plan User: courtney.cox Plotted: Jun 18, 2024 - 1:12pm Xref's: Con Docs.dwg Layout: C5.0 - UTILITY PLAN

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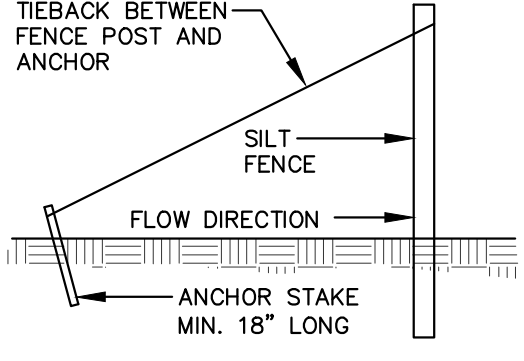
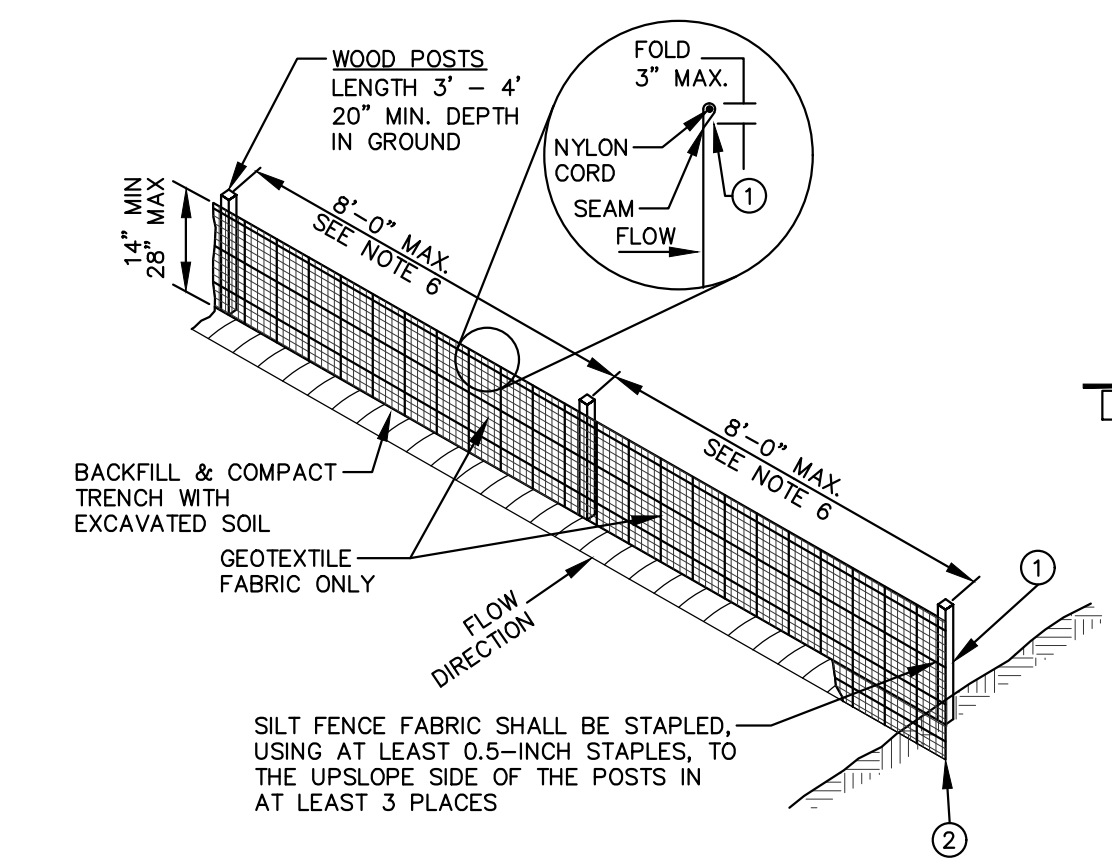


SILT FENCE ALONG SLOPES & OUTFALLS

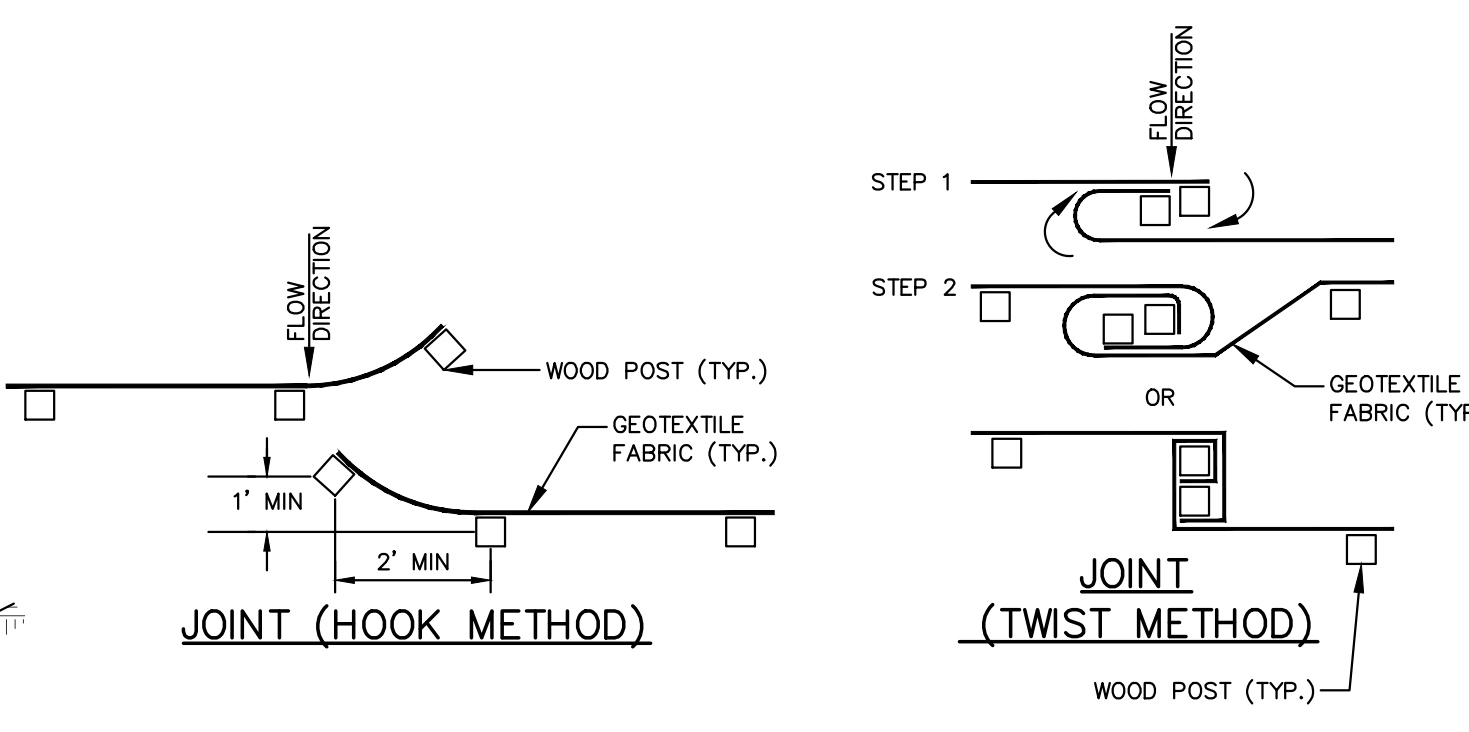


NOTE:
ADDITIONAL POST DEPTH OR TIE BACKS MAY BE REQUIRED IN UNSTABLE SOILS.

TRENCH DETAIL



SILT FENCE TIE BACK
(WHEN ADDITIONAL SUPPORT REQUIRED)



JOINT (HOOK METHOD)

JOINT (TWIST METHOD)

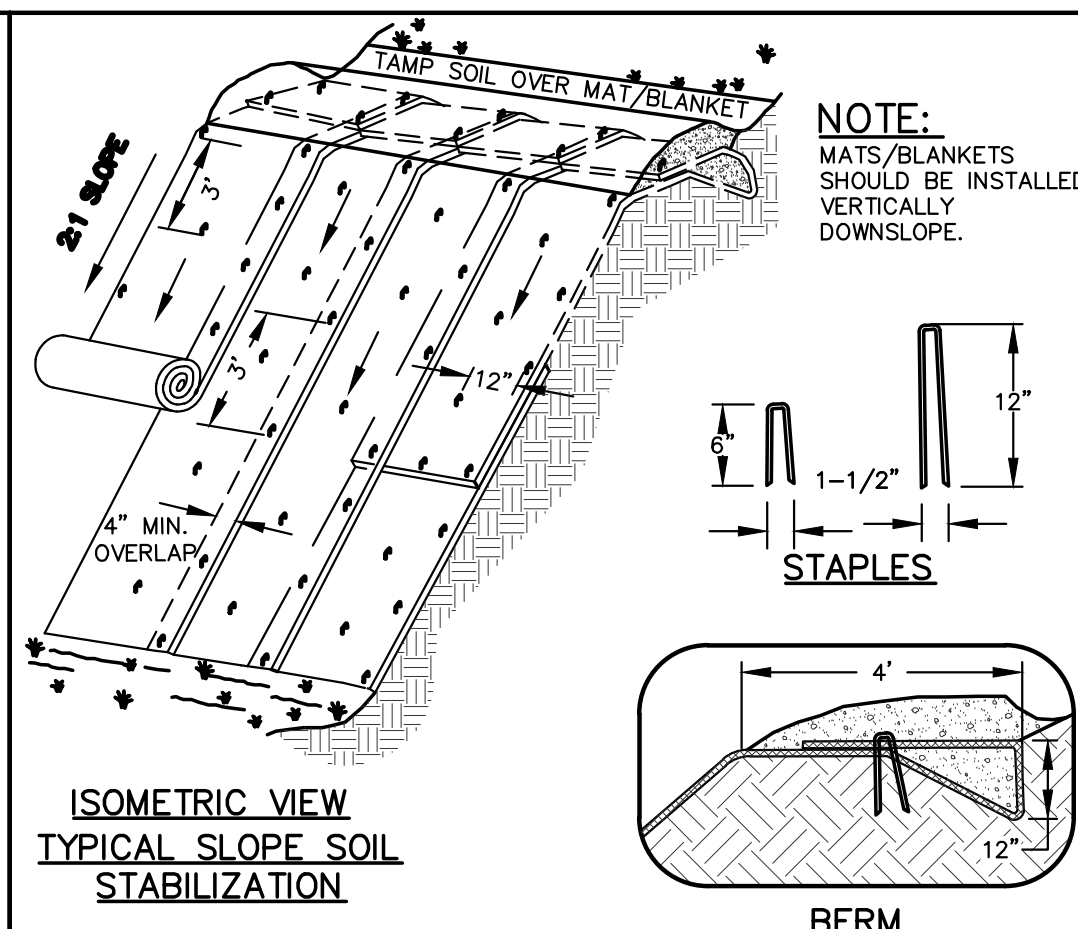
GENERAL NOTES:

- SILT FENCE SHALL BE ANCHORED BY SPREADING AT LEAST 8-INCHES OF FABRIC IN A 4-INCH WIDE AND 6-INCH DEEP TRENCH OR 6-INCH DEEP V-TRENCH ON THE UPSLOPE SIDE OF THE FENCE. TRENCHES SHALL NOT BE EXCAVATED WIDER OR DEEPER THAN NECESSARY FOR PROPER INSTALLATION.
- FOLD MATERIAL TO FIT TRENCH AND BACKFILL AND COMPACT TRENCH WITH EXCAVATED SOIL.
- WOOD POSTS SHALL BE A MINIMUM SIZE OF 1.125-INCHES x 1.125-INCHES OF DRIED OAK OR HICKORY.
- SILT FENCE TO EXTEND ABOVE THE TOP OF PIPE.
- SILT FENCE CONSTRUCTION AND GEOTEXTILE FABRIC SHALL CONFORM TO WDNR TECHNICAL STANDARD 1056.
- POST SPACING SHALL BE SELECTED BASED ON GEOTEXTILE FABRIC (8- FEET FOR WOVEN & 3- FEET FOR NON-WOVEN)

SILT FENCE

N.T.S.

REV. 7-01-2019



ISOMETRIC VIEW TYPICAL SLOPE SOIL STABILIZATION

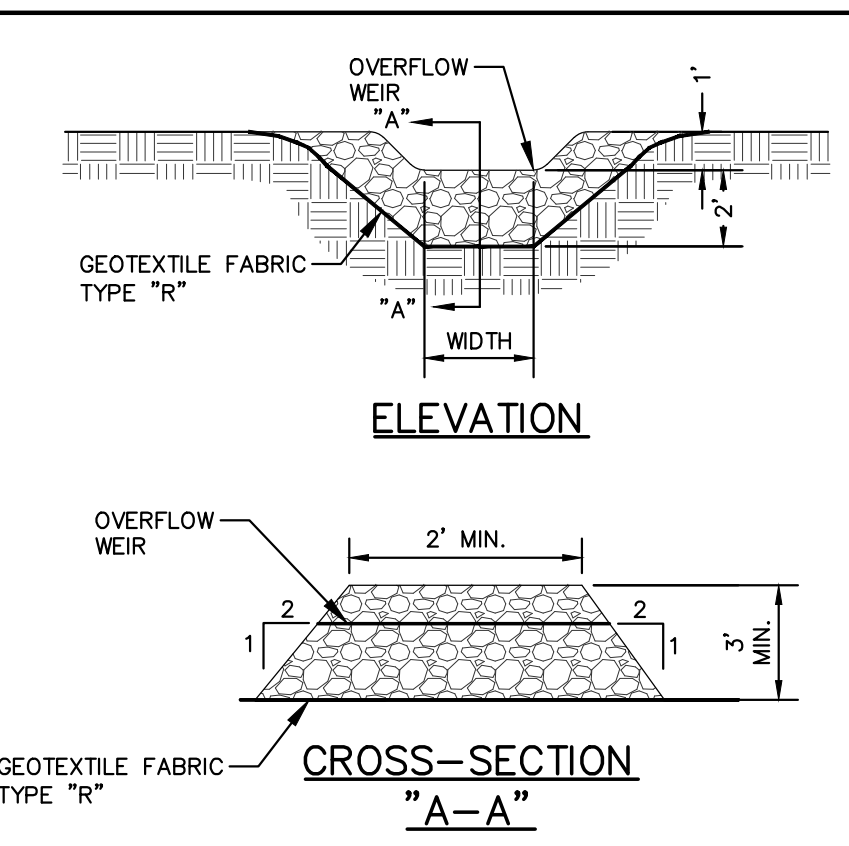
GENERAL NOTES:

- EROSION MAT CONSTRUCTION SHALL BE IN ACCORDANCE WITH WISCONSIN DNR TECHNICAL STANDARD 1052 "NON-CHANNEL EROSION MAT".
- ONLY WISDOT EROSION CONTROL PRODUCT ACCEPTABILITY LIST (PAL) APPROVED MATS SHALL BE ALLOWED. REFER TO EROSION CONTROL PLAN FOR EXACT MAT CLASSIFICATION.
- APPLY PERMANENT SEEDING BEFORE PLACING BLANKETS.
- LAY BLANKETS LOOSELY AND STAKE OR STAPLE TO MAINTAIN DIRECT CONTACT WITH THE SOIL. DO NOT STRETCH.

EROSION MATTING

N.T.S.

REV. 11-19-2018



ELEVATION

CROSS-SECTION "A-A"

GENERAL NOTES:

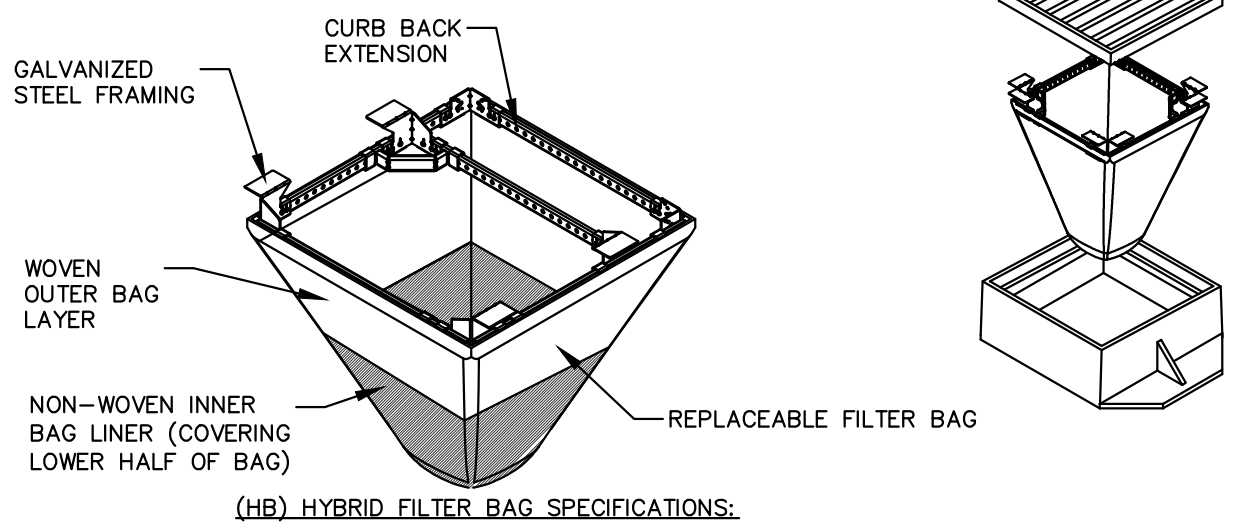
- STONE DITCH CHECKS SHALL BE CONSTRUCTED OF A WELL-GRADED ANGULAR STONE, A D50 OF 3 INCH DIAMETER OR GREATER, SOMETIMES REFERRED TO AS BREAKER RUN OR SHOT ROCK.
- EXTEND DITCH CHECK ACROSS CHANNEL OPENING TO MEET SWALE/DITCH SIDE SLOPES. WHERE CHANNEL IS LESS THAN 3 FEET DEEP, REDUCE WEIR HEIGHT TO 1 FOOT AND INCREASE THE TOP OF DITCH CHECK FROM 2 FEET TO 4 FEET.

TEMPORARY DITCH CHECK (STONE)

N.T.S.

REV. 7-31-2018

FLEXSTORM INLET FILTERS TO MEET DANE COUNTY EROSION CONTROL STANDARDS



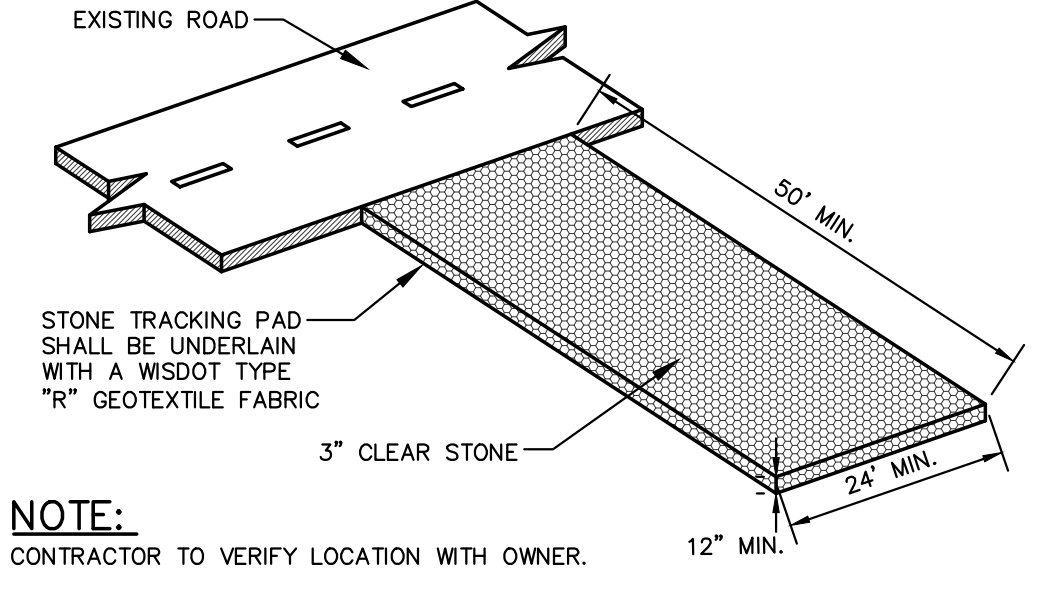
- INSTALLATION INSTRUCTIONS:**
- REMOVE GRATE FROM THE DRAINAGE STRUCTURE
 - CLEAN STONE AND DIRT FROM LEDGE (LIP) OF DRAINAGE STRUCTURE
 - DROP THE INLET FILTER THROUGH THE CLEAR OPENING SUCH THAT THE HANGERS REST FIRMLY ON THE LIP OF THE STRUCTURE
 - REPLACE THE GRATE AND CONFIRM IT IS NOT ELEVATED MORE THAN 1/8"

- MAINTENANCE GUIDELINES:**
- EMPTY THE SEDIMENT BAG IF MORE THAN HALF FILLED WITH SEDIMENT AND DEBRIS
 - REMOVE THE GRATE, ENGAGE THE LIFTING POINTS, AND LIFT FILTER FROM THE DRAINAGE STRUCTURE.
 - DISPOSE OF SEDIMENT AND DEBRIS BY THE ENGINEERING OR MAINTENANCE CONTRACT 4. ALTERNATIVELY, AN INDUSTRIAL VACUUM CAN BE USED TO COLLECT SEDIMENT FROM THE FILTER BAG

FRAMED INLET PROTECTION

N.T.S.

REV. 7-01-2019

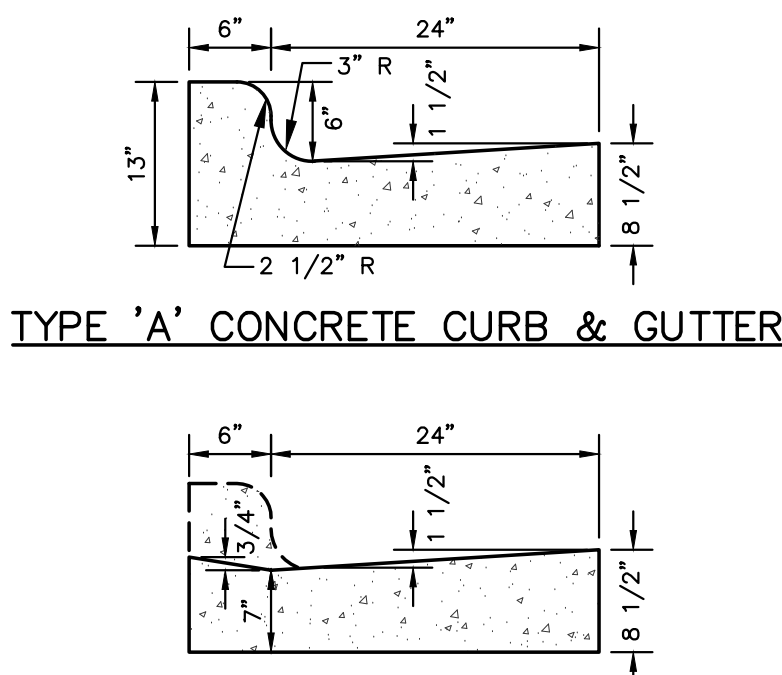


NOTE:
CONTRACTOR TO VERIFY LOCATION WITH OWNER.

CONSTRUCTION ENTRANCE

N.T.S.

REV. 12-10-2018



TYPE 'A' CONCRETE CURB & GUTTER

DRIVEWAY SECTION TYPE 'A' CONCRETE CURB & GUTTER

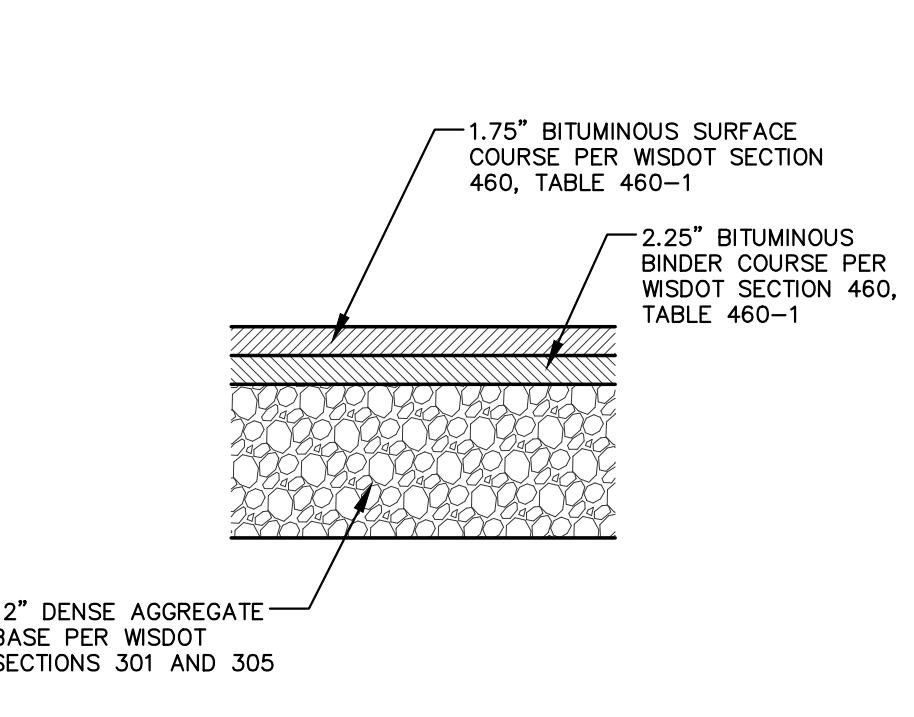
GENERAL NOTES:

- LATERAL CONTRACTION JOINTS SHALL BE PLACED AT INTERVALS OF NOT MORE THAN 15' NOR LESS THAN 6' IN LENGTH. THE JOINTS SHALL BE A MINIMUM OF 3" IN DEPTH.
- EXPANSION JOINTS SHALL BE PLACED TRANSVERSELY AT RADIUS POINTS ON CURVES OF RADIUS 200' OR LESS, AND AT ANGLE POINTS, OR AS DIRECTED BY THE ENGINEER. THE EXPANSION JOINT SHALL BE A ONE PIECE ASPHALTIC MATERIAL HAVING THE SAME DIMENSIONS AS CURB & GUTTER AT THAT STATION AND BE 1/2" THICK.
- IN ALL CASES, CONCRETE CURB & GUTTER SHALL BE PLACED ON THOROUGHLY COMPACTED CRUSHED STONE.

30" DRIVEWAY SECTION CURB & GUTTER

N.T.S.

REV. 11-20-2018



STANDARD ASPHALT PAVEMENT SECTION - OAK MEADOW DRIVE

GENERAL NOTES:

- WSDOT STANDARD SPECIFICATIONS FOR HIGHWAY AND STRUCTURE CONSTRUCTION, INCLUDING SUPPLEMENTAL SPECIFICATIONS, COMPACTION REQUIREMENTS:
- BITUMINOUS CONCRETE: REFER TO SECTION 460-3.
- BASE COURSE: REFER TO SECTION 301.3.4.2, STANDARD COMPACTION.

PAVEMENT SECTIONS

N.T.S.

REV. 7-01-2019

CATCH-IT INLET FILTER (Temporary Inlet Protection)							
Neenah Casting	Inlet Type	Grate Size	Opening Size	Bag Cap (ft)	Flow Ratings (CFS)	ADS P/N	
					HB (Hybrid Bag)	Bypass	
3067	Curb Box	35.25 x 17.75	33.0 x 15.0	4.4	2.0	5.8	62LCBEXTHB
3246A	Curb Box	35.75 x 23.875	33.5 x 21.0	4.2	1.1	3.3	62LCB3624HB
3030	Square/Rect (SQ)	23 x 18	20.5 x 13.5	1.6	0.7	2.2	62MCR2316HB
3067-C	Square/Rect (SQ)	35.25 x 17.75	33 x 15	3.2	1.0	5.2	62LSQ3618HB
R-2501	Round (RD)	-26	-24	2.3	0.8	5.2	62MRD26HB
R-1772/2560	Round (RD)	22.25-23.5	20.5-21	1.5	0.6	4.6	62MRD22HB

Woven and Non-Woven Geotextile Filter Bag Properties (Minimum Average Roll Values)			
PROPERTY	TEST METHOD	WOVEN (OUTER)	NON-WOVEN (LINER)
TENSILE STRENGTH	ASTM D4632	350 x 225 lbs	100 lbs
ELONGATION	ASTM D4632	20% x 15%	50%
CBR PUNCTURE	ASTM D6241	1000 lbs	65 lbs
TRAPEZOIDAL TEAR	ASTM D4538	110 x 75 lbs	45 lbs
UV RESISTANCE	ASTM D4355	90%	70%
OPENING SIZE (AOS)	ASTM D4751	20 US STD SIEVE	40 US STD SIEVE
PERMEABILITY	ASTM D4491	1.5 sec ⁻¹	2.0 sec ⁻¹
WATER FLOW RATE	ASTM D4491	200 gal/min/ft ²	145 gal/min/ft ²
MINIMUM FILTER BAG VOLUME		2 CUBIC FT	



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MADISON REGIONAL OFFICE
507 WEST VERONA AVENUE, SUITE 500
VERONA, WISCONSIN 53593
P. 608.848.5060

CLIENT:
THOMPSON ENGINEERING LLC

CLIENT ADDRESS:
**918 DEMING WAY, FLOOR 3
MADISON, WI 53717**

PROJECT:
LOT 115 OAK MEADOWS

PROJECT LOCATION:
**2621 OAK MEADOWS DRIVE
FITCHBURG, DANE COUNTY
WI, 53711**

PLAN MODIFICATIONS:		
#	Date:	Description:
1	05.10.24	P-PLAT SUBMITTAL
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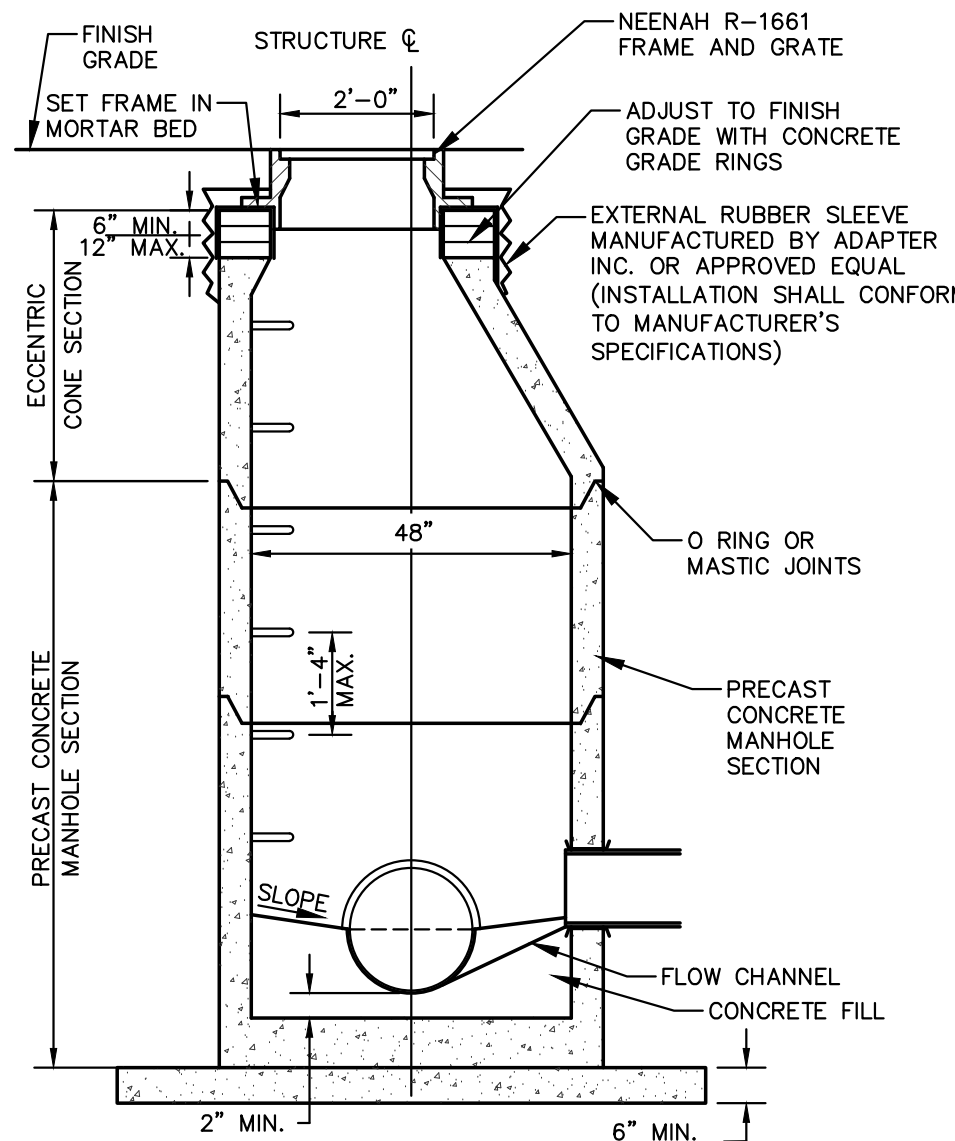
Designed By: CAC/CHG
Reviewed By: CWD
Approved By: MRH

DETAILS

SHEET NUMBER:

C6.0

JSD PROJECT NO: 23-13509



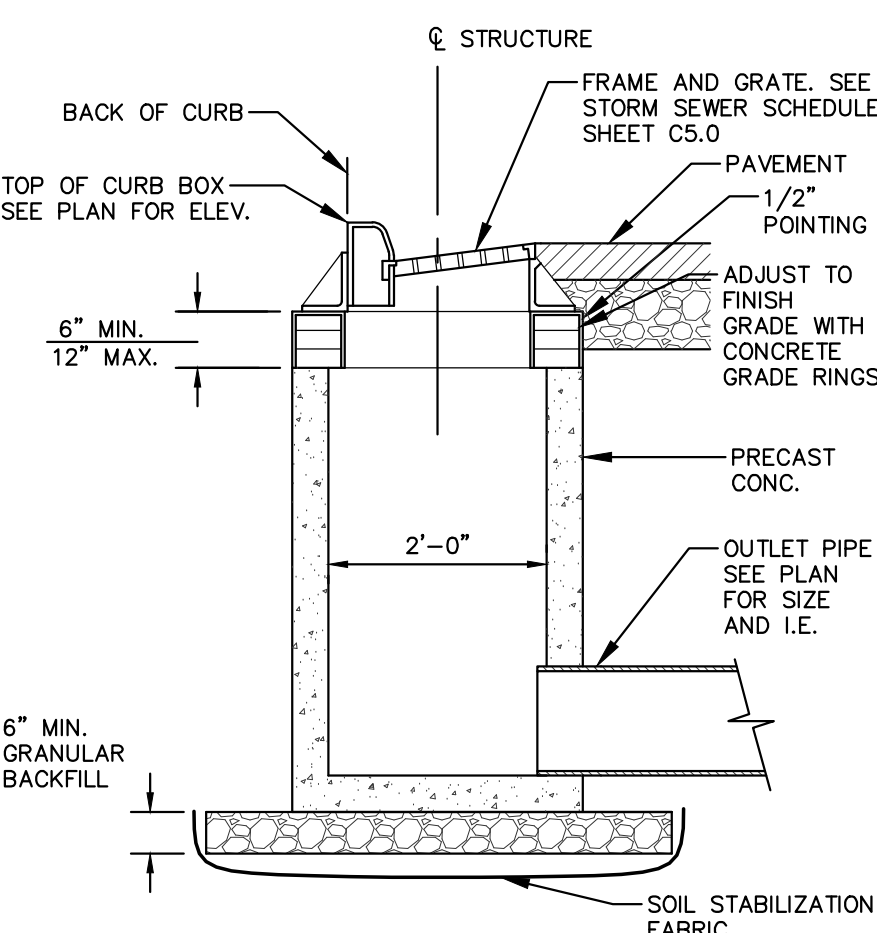
GENERAL NOTES:

1. MANHOLE CONSTRUCTION TO MEET REQUIREMENTS OF ASTM C478.
2. JOINTS SHALL BE WATERTIGHT. USE BUTYL RUBBER GASKET.
3. USE MORTAR FOR PIPE CONNECTIONS.
4. ALL MANHOLES SHALL HAVE RUBBER CHIMNEY BOOT SEALS.

SANITARY SEWER MANHOLE

N.T.S.

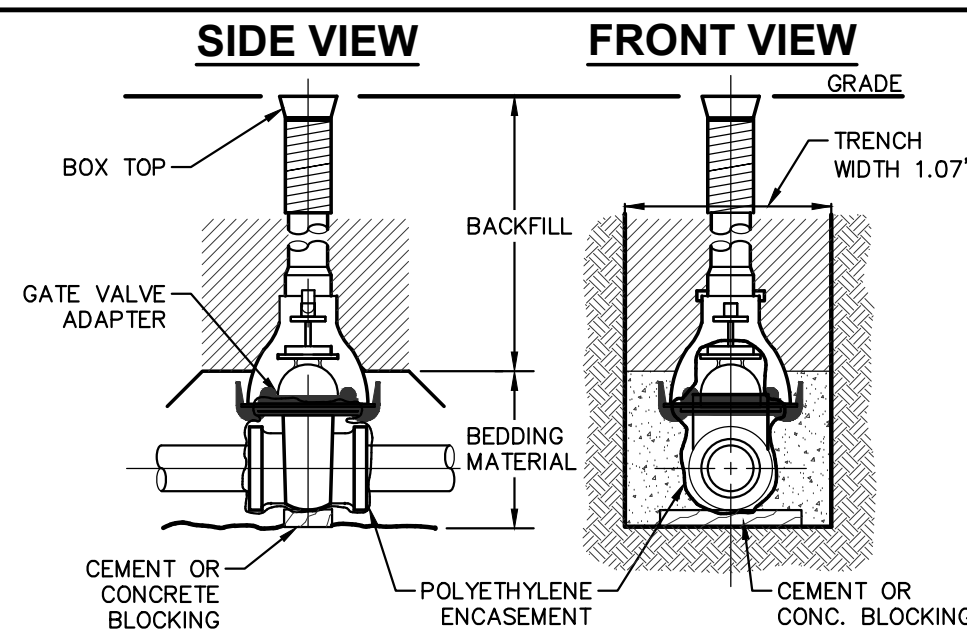
REV. 12-6-2018



STORM INLET

N.T.S.

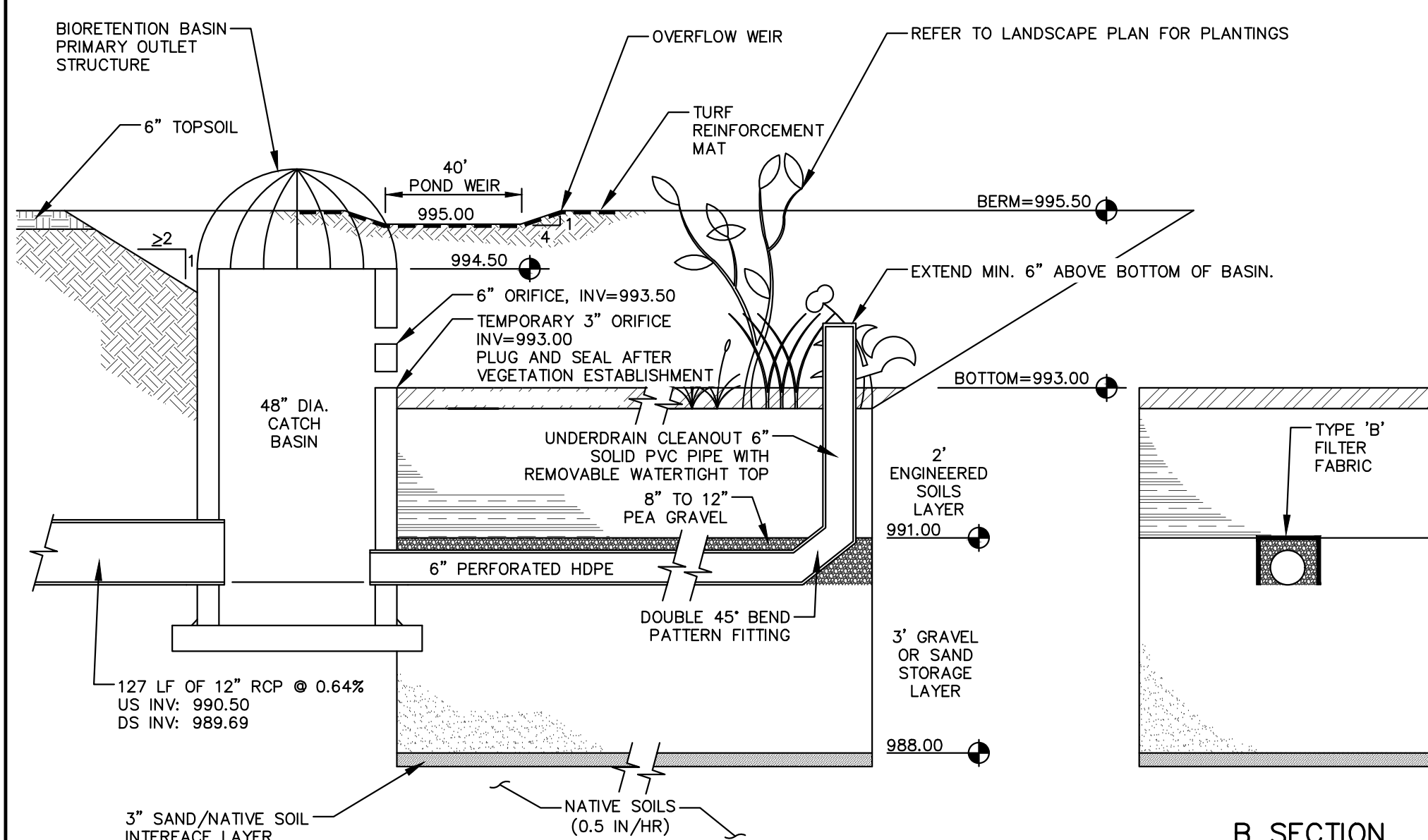
REV. 11-26-2018



GATE VALVE BOX SETTING

N.T.S.

REV. 12-04-2018



B SECTION

GENERAL NOTES:

1. ALL CONSTRUCTION PRACTICES SHALL MEET THE SPECIFICATIONS OF THE WDNR TECHNICAL STANDARD 1004 - BIORETENTION FOR INFILTRATION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN A COPY OF THIS STANDARD AND CONSTRUCT THE BIORETENTION DEVICE IN ACCORDANCE WITH THE REQUIREMENTS OUTLINED THEREIN.
2. CONTRACTOR SHALL INSTALL 24" OF ENGINEERED SOIL CONSISTING OF: 85% ASTM C33 SAND, 15% CERTIFIED COMPOST (SEE GENERAL NOTE 3).
3. CERTIFIED COMPOST SHALL CONSIST OF: >40% ORGANIC MATTER, <60% ASH CONTENT, pH OF 6-8, AND MOISTURE CONTENT OF 35-50% BY WEIGHT.
4. SAND/GRAVEL STORAGE LAYER SHALL CONSIST OF SAND OR GRAVEL MATERIAL MEETING THE SPECIFICATIONS IN SECTION V.B.7 OF WDNR TECHNICAL STANDARD 1004.
5. SAND/NATIVE SOIL INFILTRATION LAYER SHALL BE FORMED BY A LAYER OF SAND 3 INCHES DEEP, WHICH IS VERTICALLY MIXED WITH THE NATIVE SOIL TO A DEPTH OF 2-4 INCHES.
6. CONFIRM WITH GEOTECHNICAL ENGINEER THAT THE SILT LOAM SOIL PROFILE HAS BEEN REACHED PRIOR TO BACKFILLING THE BIORETENTION BASIN. DEEP TILL MINIMUM 2 FEET OF NATIVE SOIL TO PROMOTE INFILTRATION.
7. IF ADDITIONAL EXCAVATION IS REQUIRED BELOW THE SAND SOIL PROFILE TO REACH THE LISTED NATIVE SOIL LAYER, THE BACKFILL USED TO RETURN THE BOTTOM OF THE BIORETENTION SYSTEM TO THE BOTTOM OF THE SAND LAYER ELEVATION MUST HAVE AN EQUAL OR HIGHER INFILTRATION RATE THAN THE LISTED NATIVE SOIL LAYER AS CONFIRMED BY A GEOTECHNICAL ENGINEER.
8. FILTER FABRIC SHALL BE PLACED ABOVE AND ON THE SIDES OF THE PERFORATED PIPE, BETWEEN THE PEA GRAVEL AND THE ENGINEERED SOIL, A WIDTH OF 4 FEET CENTERED OVER THE FLOW LINE OF THE PIPE.
10. ANNUAL RYE GRASS SHALL BE SEEDED AT 40 LB/ACRE WITH THE SEED MIX IN THE AREAS SURROUNDING THE BASIN, ON SIDE SLOPES, AND OVER ANY LAND THAT DISCHARGES INTO THE BASIN FOR EROSION CONTROL. WHEN BASIN IS BROUGHT ON-LINE, ROOTSTOP AND PLUGS ARE REQUIRED TO ESTABLISH VEGETATION AT THE INVERT OF THE BASIN.
11. RUNOFF MUST INFILTRATE WITHIN 24-HOURS. BASINS UNABLE TO MAINTAIN THESE RATES MUST BE DEEP TILLED, REGRADED, AND IF NECESSARY REPLANTED TO RESTORE ORIGINAL INFILTRATION RATES.
12. ALL WORK TO BE CONDUCTED IN CONFORMANCE WITH APPLICABLE LOCAL, REGIONAL, AND STATE STORMWATER STANDARDS FOR THE PROJECT SITE AS APPROVED BY THE REGULATORY ENGINEER.
13. SEE LANDSCAPING PLAN AND CONSULT WITH LANDSCAPE ARCHITECT OR ECOLOGICAL PLANTING AGENCY FOR APPROPRIATE SEED MIX, PLANTS AND PLANTING CONFIGURATIONS.

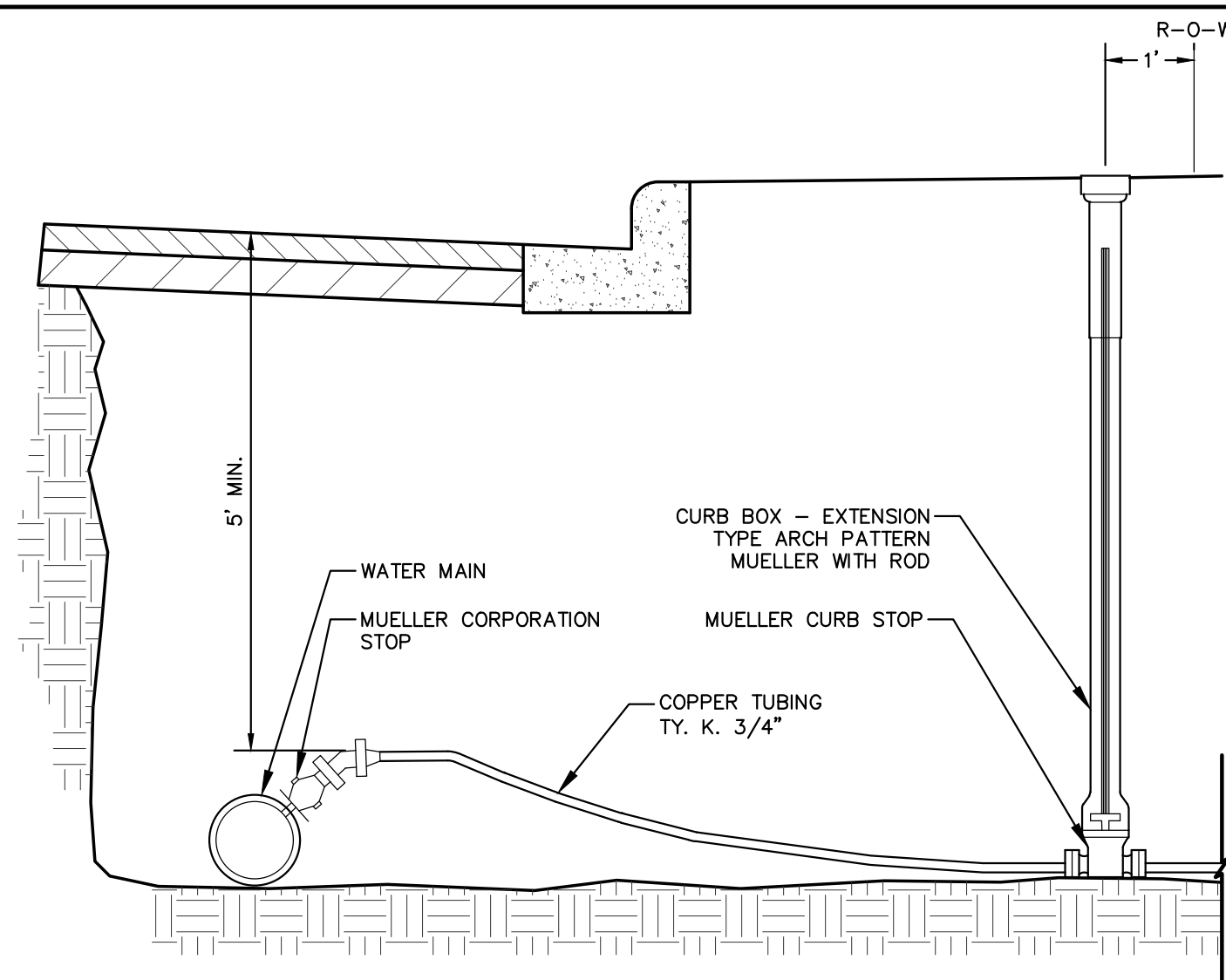
NOTE: INFILTRATION DEVICES ARE DESIGNED IN ACCORDANCE WITH THE SPECIFICATIONS OF THE WISCONSIN DEPARTMENT OF NATURAL RESOURCES (WDNR), COUNTY, MUNICIPALITY, AND ENGINEERING STANDARD OF CARE. ALL DESIGNATED INFILTRATION AREAS (E.G. RAIN GARDENS, INFILTRATION BASINS, BIORETENTION DEVICES) SHALL BE FENCED PRIOR TO CONSTRUCTION AND REMAIN UNDISTURBED AND PROTECTED DURING THE CONSTRUCTION OF PROPOSED SITE IMPROVEMENTS. PROPOSED BIORETENTION DEVICES SHALL NOT BE CONSTRUCTED UNTIL THE DEVICE'S CONTRIBUTING WATERSHED AREA MEETS ESTABLISHED VEGETATION REQUIREMENTS SET FORTH WITHIN THE RESPECTIVE WDNR TECHNICAL STANDARDS. IF THE LOCATION OF THE INFILTRATION AREA CONFLICTS WITH CONSTRUCTION STAGING AND/OR CONSTRUCTION TRAFFIC AND IS DISTURBED, COMPACTION MITIGATION WILL BE REQUIRED AT THE CONTRACTOR'S EXPENSE.

THE CONTRACTOR IS REQUIRED TO PROVIDE QUALIFIED STAFF FOR INSPECTION AND OBSERVATION OF THE CONSTRUCTION ACTIVITIES RELATING TO ALL JOB SITE REGULATORY COMPLIANCE INCLUDING THE PROTECTION AND CONSTRUCTION OF ALL STORMWATER MANAGEMENT FEATURES. ANY OBSERVATION OF PLAN OR SITE DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER OF RECORD PRIOR TO CONSTRUCTION.

BIORETENTION BASIN

N.T.S.

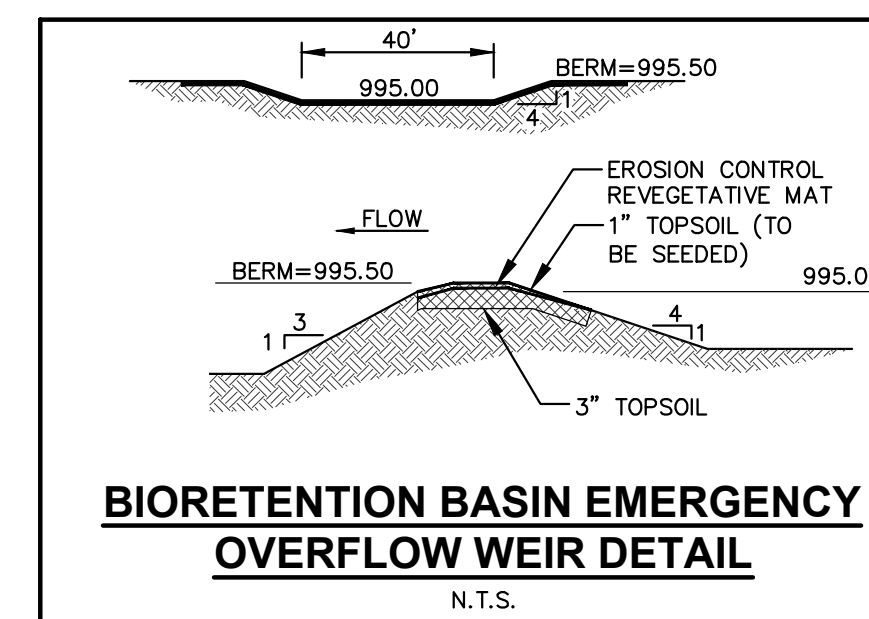
REV. 12-7-2018



WATER SERVICE INSTALLATION

N.T.S.

REV. 12-04-2018



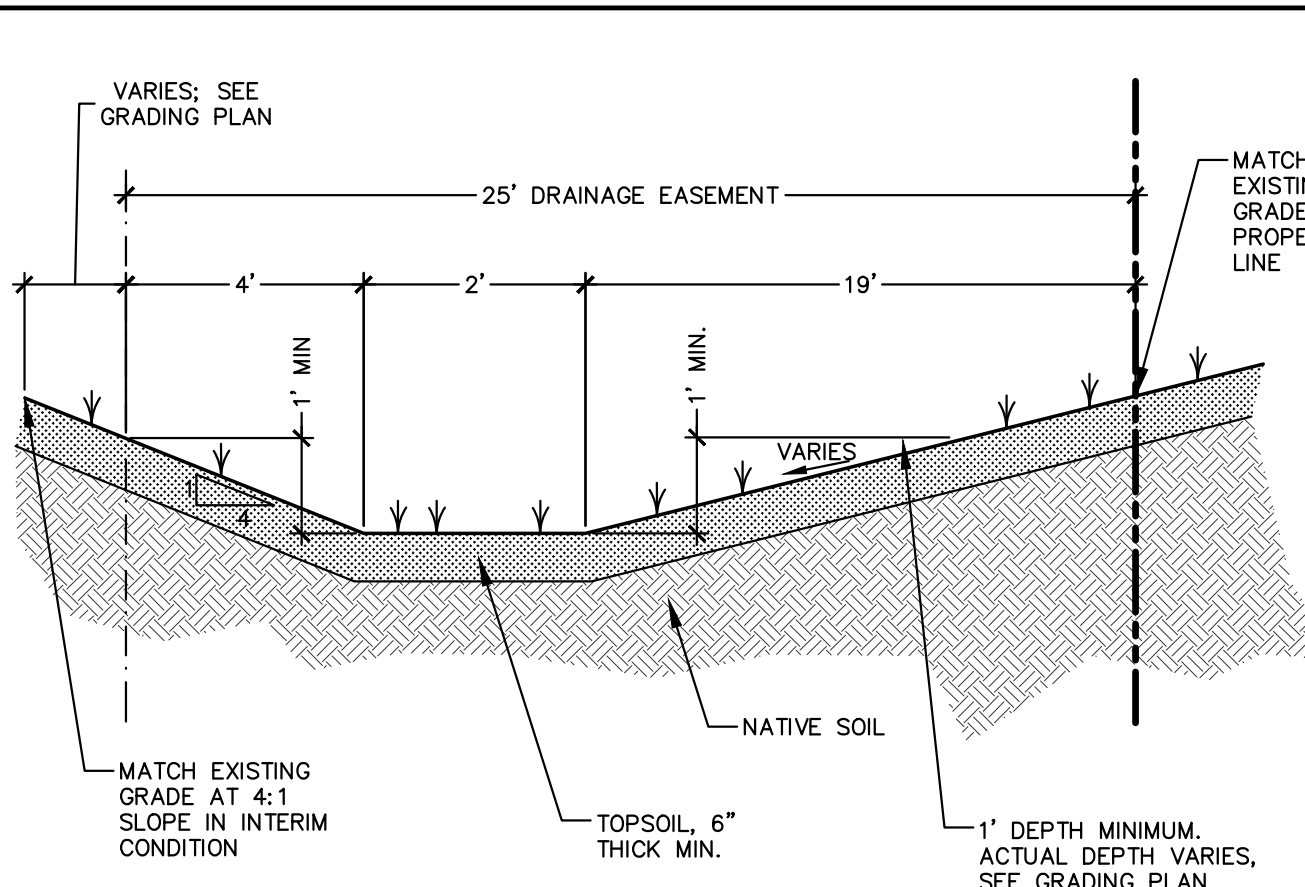
BIORETENTION BASIN EMERGENCY OVERFLOW WEIR DETAIL

N.T.S.

THE STORMWATER MANAGEMENT FEATURES CONTAINED WITHIN THIS PLAN SET HAVE BEEN DESIGNED IN ACCORDANCE WITH APPLICABLE STANDARDS SET FORTH IN WISCONSIN DNR NR151 AND LOCAL ORDINANCES. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE PROPER CONSTRUCTION PRACTICES HAVE BEEN UTILIZED AND THAT STORMWATER MANAGEMENT FEATURES HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH APPROVED DESIGN PLANS. JSD PROFESSIONAL SERVICES, INC. (JSD) SHALL NOT BE LIABLE FOR ANY CONSTRUCTION PRACTICES OR INSTALLATION WHICH DEVIATES FROM THE APPROVED PLAN SET. ONCE THE OWNER HAS PROVIDED FINAL APPROVAL TO THE WORK PERFORMED BY THE CONTRACTOR AND ENSURED COMPLIANCE WITH THE PLAN, IT IS THE OWNER'S RESPONSIBILITY TO MAINTAIN STORMWATER MANAGEMENT FEATURES IN ACCORDANCE WITH THE RECORDED MAINTENANCE AGREEMENT. PROPER OPERATION IS DEPENDENT ON A MULTITUDE OF VARIABLES INCLUDING WEATHER. THESE COMPONENTS REQUIRE ONGOING MAINTENANCE FOR WHICH THE OWNER IS RESPONSIBLE. JSD TAKES NO RESPONSIBILITY FOR PROPER OPERATION OF THE WATER QUALITY COMPONENTS.

SAND STORAGE LAYER: IF NATIVE SOIL INFILTRATION RATES ARE GREATER THAN OR EQUAL TO THE DESIGN SAND LAYER (3.6 IN/HR), NATIVE SOILS MAY BE USED. GEOTECHNICAL CONSULTANT SHALL PROVIDE THIS INFORMATION IN WRITTEN DOCUMENTATION FOR VERIFICATION PRIOR TO CONSTRUCTION.

AS-BUILT SURVEY AND CERTIFICATION: UPON CONSTRUCTION COMPLETION AND STABILIZATION, AN AS-BUILT SURVEY IS TO BE CONDUCTED FOR BASIN AND CERTIFIED BY THE ISSUING ENGINEER. SURVEYOR IS TO CONFIRM THE TEMPORARY 3" ORIFICE IN THE BIORETENTION BASIN OUTLET HAS BEEN PLUGGED AND SEALED. AS-BUILT PLANS ARE TO BE SUBMITTED TO MUNICIPALITY FOR FINAL APPROVAL.



REAR YARD DRAINAGE SWALE CROSS-SECTION

N.T.S.



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MADISON REGIONAL OFFICE
507 WEST VERONA AVENUE, SUITE 500
VERONA, WISCONSIN 53593
P. 608.848.5060

CLIENT:
THOMPSON ENGINEERING LLC

CLIENT ADDRESS:
**918 DEMING WAY, FLOOR 3
MADISON, WI 53717**

PROJECT:
LOT 115 OAK MEADOWS

PROJECT LOCATION:
**2621 OAK MEADOWS DRIVE
FITZBURG, DANE COUNTY
WI, 53711**

PLAN MODIFICATIONS:

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Designed By: CAC/CHG
Reviewed by: CWJ
Approved By: MRH

SHEET TITLE:

DETAILS

SHEET NUMBER:

C6.1

JSD PROJECT NO: 23-13509

File: \JSD\Projects\2023\2313509\DWG\Civil\Sheets\C6.1 - DETAILS User: courney.cox Plotted: Jun 18, 2024 - 11:13pm Xrefs:

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Professional Service Industries, Inc.
821 Corporate Court, Waukesha, WI 53189
Phone: (262) 521-2125
Fax: (262) 521-2471

January 15, 2024

Thompson Engineering LLC
918 Deming Way, Floor 3
Madison, WI 53717

Attn: Mr. John Thompson
Project Engineer

Re: Geotechnical Exploration and Evaluation
Oak Meadow Drive Sanitary Sewer Installation and Bioretention Basin
NEC Oak Meadow Drive and Targhee Street
Fitchburg, WI
PSI Project No. 00523135

Dear Mr. Thompson:

The geotechnical exploration and evaluation for the referenced project has been completed. An electronic copy of the report is being provided via email. Paper copies can be issued upon request. After you have had the opportunity of reading the report, please call at any time with any questions or comments you may have. Professional Service Industries, Inc. (PSI), an Intertek Company, appreciates the opportunity to be of service on this project, and looks forward to continuing as your geotechnical consultant during the design and construction phases, as well as your upcoming projects.

Sincerely,

PROFESSIONAL SERVICE INDUSTRIES, INC.

Caige Tubic
Staff Geologist

James M. Becco, P.E.
Regional Vice President

Patrick J. Patterson, P.E., P.G.
Senior Engineer



**GEOTECHNICAL ENGINEERING
SERVICES REPORT**

For the:

Oak Meadow Drive Sanitary Sewer Installation
and Bioretention Basin
NEC Oak Meadow Drive and Targhee
Street
Fitchburg, WI

Prepared for:

Thompson Engineering LLC
918 Deming Way, Floor 3
Madison, WI 53717

Prepared by:

Professional Service Industries, Inc.
821 Corporate Court, Waukesha, WI 53189
Phone: (262) 521-2125
Fax: (262) 521-2471

January 15, 2024

PSI Report Number: 00523337



Handwritten signature of Caige Tubic in black ink.

Caige Tubic
Staff Geologist

Handwritten signature of Patrick J. Patterson in black ink.

Patrick J. Patterson,
P.E., P.G.
Senior Engineer



Handwritten signature of James M. Becco in black ink.

James M. Becco, P.E.
Regional Vice President
Geotechnical Services

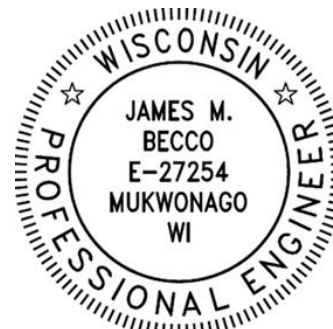


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APPENDIX

- Figure 1 – Boring/Test Pit Location Plan
- Soil Boring/Test Pit Logs
- General Notes
- Soil Evaluation – Storm Form
- USDA Classification Chart





INTRODUCTION

General

This report presents the results of the subsurface exploration for the proposed Oak Meadow sanitary sewer relocation and Bioretention Basin construction project in Fitchburg, Wisconsin. The work was performed for Thompson Engineering LLC, at the request of Mr. John Thompson.

Purpose

The purpose of this study was to evaluate the subsurface conditions at specific boring locations and to establish parameters for use by the engineers in preparing the bioretention and utility installation designs for the proposed project.

Scope

The scope of services included geotechnical exploration, an evaluation of soil characteristics by field and laboratory testing, and an evaluation of the data obtained. Subgrade preparation recommendations and construction considerations are also provided. The scope of the field work, including the number and depth of the borings was determined by the client in consultation with PSI.

Authorization

The description of services and authorization to perform this subsurface exploration and evaluation were in the form of signed PSI Proposal No. 414250, dated December 12, 2023. The general conditions for the performance of the work were referenced in the proposal. This report has been prepared on behalf of, and exclusively for the use of Thompson Engineering LLC. The information contained in this report may not be relied upon by any other parties without the express written consent of PSI, and acceptance by such parties of PSI's General Conditions.

SITE AND PROJECT DESCRIPTION

Site Features

The proposed project area is located to the northeast of the intersection of Oak Meadow Drive and Targhee Street in Fitchburg, Wisconsin. At the time of the exploration the site was occupied by the eastern half of Oak Meadow Park. The surface was covered with grass and trees. Aerial photos viewed on Google Earth from various years between 1992 and 2022 indicate that the subject site was agricultural land at the time of the 1992 photo. In the 2004 photo, it appeared that some grading activities had occurred on the southern portion of the site, possibly associated with street construction. It also appears that grading occurred on other portions of the site in subsequent photos through 2009. In the 2010 photo, the site is similar in appearance to as it is currently. However, several of the photos were extremely grainy and details were



difficult to discern. Surrounding the site were residential homes to the south and east, an agricultural field to the north and Oak Meadow Drive to the west of the site. The subject site is depicted on the enclosed Boring Location Plan (Figure 1). The topography of the site slopes down to the north with a difference in elevation between the borings and test pits of about 28 feet (EL. 1020 to EL. 992).

Project Description

Based on the information provided, the proposed project consists of relocation of the existing new sanitary sewer and construction of a new bioretention basin. The new sanitary sewer utility is planned to be installed by open cut trenching. The pipe is planned to be 8 inches in diameter, bearing at depths of about 14 feet (EL. 1006) below existing grade at SB-1 (EL. 1020) and about 8 feet (EL. 997) below grade at SB-2 (EL. 1005).

It is understood that a stormwater management area will be located on the northern portion of the site. The area is planned to be a bioretention basin. The size and bottom elevation were not provided.

When additional information regarding the project becomes available, and/or if any of the information discussed herein differs from current plans or changes as design progresses, PSI must be informed so that any necessary revisions to this report can be made.

EXPLORATION AND LABORATORY PROCEDURES

Scope Summary

The field and laboratory data utilized in the evaluation of the subsurface materials was obtained by drilling exploratory test borings, securing soil samples by the split-spoon sampling method, and subjecting the samples to standard laboratory testing.

Field Exploration

Two (2) soil test borings (SB-1 and SB-2) were performed in the area of the proposed new sanitary sewer line. The borings were drilled to a planned depth of about 15 feet below existing grade. Two (2) test pits (TP-1 and TP-2) were performed in the area of the proposed new bioretention basin to a planned depth of about 15 feet. SB-2 was offset about 50 feet northwest of the planned location due to dense trees. The number, depth, and location of the borings and test pits were determined by the client. The borings and test pits were located in the field by PSI utilizing a consumer grade GPS device. They are estimated to be accurate to within several feet. The approximate locations are shown on the Boring Location Plan (Figure 1), which is provided in the Appendix of this report. The surface elevations shown on the logs were provided by the client.

The soil test borings were performed with an all-terrain vehicle (ATV) mounted rotary drilling rig utilizing continuous flight hollow stem augers to advance the holes. Representative samples



were obtained by the Standard Penetration Test (SPT) method using split-spoon sampling in general accordance with ASTM D-1586 procedures. Samples were generally collected at 2.5-foot intervals to 10 feet, and then at 5-foot intervals thereafter to the end of the borings. The standard penetration value (N) is defined as the number of blows of a 140-pound hammer, falling thirty (30) inches, required to advance the split-spoon sampler one (1) foot into the soil. The sampler is lowered to the bottom of the drill hole and the number of blows recorded for each of the three (3) successive increments of six (6) inches penetration. The "N" value is obtained by adding the second and third incremental numbers. The SPT provides a means of estimating the relative density of granular soils and comparative consistency of cohesive soils, thereby providing a method of evaluating the relative strength and compressibility characteristics of the subsoils.

The test pits were performed with a backhoe and soil samples were collected by hand at the approximate intervals shown on the logs. After completion of the test pits, the excavations were backfilled (by loose dumping and tamping with the backhoe bucket) to the ground surface with the excavation spoils. As a result, the relatively loose fill soils are not suitable for the support of foundations, slabs, utilities, or other structures.

The SPT soil samples were transferred into clean glass jars immediately after retrieval and returned to the laboratory upon completion of the field operations. Samples will be discarded unless other instructions are received. The soil samples were visually classified in general accordance with the Unified Soil Classification System (ASTM D- 2488-75). The soil samples obtained from proposed stormwater management area test pits were visually classified in general accordance with USDA textural soil classification procedures. A description of the subsurface conditions encountered at each boring location is shown on the enclosed Soil Boring Logs. After completion of the borings, the auger holes were backfilled to the ground surface with bentonite chips.

A copy of the Soil Boring/Test Pit Logs and Boring/Test Pit Location Plan (Figure 1) are enclosed in the Appendix. The soil stratification shown on the logs represents the approximate soil conditions in the actual boring locations at the time of the exploration. The terms and symbols used on the logs are described in the General Notes found in the Appendix.

Laboratory Physical Testing

Soil samples obtained from the exploration were visually classified in the laboratory, and subjected to testing, which included moisture content determinations. The values of strength tests performed on soil samples obtained by the Standard Penetration Test Method (SPT) are considered approximate, recognizing that the SPT method provides a representative but somewhat disturbed soil sample.

The laboratory testing was performed in general accordance with the respective ASTM methods, as applicable, and the results are shown on the boring logs in the Appendix.



DESCRIPTION OF SUBSURFACE CONDITIONS

General

A description of the subsurface conditions encountered at the test boring and test pit locations is shown on the Soil Boring/Test Pit Logs. The lines of demarcation shown on the logs represent an approximate boundary between the various soil classifications; however, some variation is expected. It must be recognized that the soil descriptions are considered representative estimates for the specific test hole location, but those variations may occur between and beyond the sampling intervals and boring locations. Soil depths, and layer thicknesses, and demarcation lines can be utilized for preconstruction planning, but should not be expected to yield exact and final quantities. A summary of the major soil profile components is described in the following paragraphs.

Soil Conditions

Borings (SB-1 and SB-2) - USCS Classification

The surficial materials at the borings consisted of about 8 inches of brown lean clay classified as topsoil. Beneath the topsoil, natural brown lean clay was present to a depth of about 3 feet (EL. 1017 to EL. 1002). Below the lean clay, brown to reddish brown silty fine sand was present to the termination of the borings. At SB-1, possible cobbles were present from about 2 to 15 feet (EL. 1018 to EL. 1005). The natural granular soils were medium dense to extremely dense in relative density and exhibited N-values between about 10 bpf to 80 bpf. The cohesive natural soils were in a stiff condition with an unconfined compressive strength of about 1.6 tons per square foot (tsf).

Stormwater Management Areas (TP-1 and TP-2) - USDA Classification

The surface materials encountered at the test pit locations consisted of about 4 inches to 1 foot of dark brown silt loam. Beneath the surface materials were natural soils consisting of silty clay loam, silt loam, sandy loam, and loamy sand to the termination depth.

The foregoing discussion of soil conditions on this site represents a generalized soil profile as determined at the test boring locations. A more detailed description and supporting data for each test location can be found on the individual Soil Boring Logs.

Groundwater Observations

Groundwater observations were made during the drilling and excavating operations, and in the open boreholes upon completion. Groundwater was not encountered at the time of auger advancement or upon completion of excavation, drilling, and removal of the augers. Additionally, groundwater was not encountered in the test pits.

The groundwater observations reported herein are considered approximate. It must be recognized that groundwater levels fluctuate with time due to variations in seasonal



precipitation, lateral drainage conditions, and soil permeability characteristics. Longer term monitoring would be required and is recommended to further evaluate groundwater levels on this site.

CONSIDERATIONS AND RECOMMENDATIONS

General Development Considerations

In general, the natural soils encountered in the borings can be used for support of the proposed sanitary sewer. Some difficulty with excavation and subgrade stability may be experienced during construction. A discussion of construction guidelines and recommendations is included in the following sections.

Groundwater Control

Groundwater observations were made during the drilling and excavating operations, and in the open boreholes upon completion. Groundwater was not encountered at the time of auger advancement or upon completion of excavation, drilling, and removal of the augers. Additionally, groundwater was not encountered in the test pits.

On the basis of the observations, no major difficulty with groundwater is expected within typical shallow basin and utility excavations. A gravity drainage system and filtered sump pump should be adequate to control low volume perched water. However, for rising groundwater levels, deeper excavations, or for large volume perched zones, prolonged dewatering with a series of sump pumps may be necessary to facilitate construction.

Since portions of the anticipated subgrade soils are subject to softening when exposed to free moisture, every effort should be made to keep excavations dry. Site grading should be performed to direct runoff away from the construction area, so that the potential for the softening of the subgrade soils is reduced.

While no groundwater was encountered at the time the borings were drilled or excavation, seasonal variations in precipitation, site drainage conditions, soil permeability, and other factors can cause groundwater levels to rise and to be present in the upper soils at other times of the year, including during construction.

Excavations and Site Drainage

Sloping, shoring or bracing of excavation sidewalls will be necessary to facilitate construction and to protect life and property. Sloughing and caving should be expected within unprotected excavations, especially in the presence of water. The degree of excavation instability problems is dependent upon the depth and length of time that excavations remain open, excavation bank slopes, water levels and the effectiveness of any dewatering systems. All excavation work must be performed in accordance with OSHA and local building code requirements.



All excavations must be performed with caution and utilize methods which will prevent undermining or destabilization of buildings, utilities, pavements, or other structures. The use of a properly designed shoring and bracing, sheet piling, or underpinning system must be utilized as necessary to adequately protect utilities, pavements, and other structures. This must be performed by an experienced specialty contractor. Additionally, extreme care must be used during the installation of any bracing system, especially those using driven or vibratory methods, in order to avoid damaging existing buildings, utilities, and other structures. Consideration should be given to the performance of video and/or photographic documentation of the condition of nearby buildings, utilities, and other structures prior to installation.

Since the subgrade soils are generally sensitive to moisture, every effort should be made to provide adequate drainage across the site during construction, and to prevent ponding of runoff on the subgrade. These soils are also subject to erosion caused by runoff, and erosion control measures should be implemented where needed or required by local ordinances.

It is mandated that excavations, whether they be for utility trenches, basement excavations or footing excavations, be constructed in accordance with current Occupational Safety and Health Administration (OSHA) guidelines to protect workers and others during construction. PSI recommends that these regulations be strictly enforced.

The contractor is solely responsible for designing and constructing stable, temporary excavations and should shore, slope, or bench the sides of the excavations as required to maintain stability of both the excavation sides and bottom. The contractor's "responsible person", as defined in 29 CFR Part 1926, should evaluate the soil exposed in the excavations as part of the contractor's safety procedures. In no case should slope height, slope inclination, or excavation depth, including utility trench excavation depth, exceed those specified in local, state, and federal safety regulations.

PSI is providing this information solely as a service to our client. PSI does not assume responsibility for construction site safety or the contractor's or other parties' compliance with local, state, and federal safety or other regulations.

Sanitary Sewer Installation Considerations

The soils encountered within the borings can generally be used for support of the planned sanitary sewer. However, conditions may vary along the route. Some undercutting of soft, unstable or otherwise unsuitable soils may be necessary. All pipelines and associated manholes or other structures must bear upon a suitable and stable subgrade of sufficient strength, or properly placed structural fill.

Pipe Material

In order to reduce the amount of pipe deflection, it must be recognized that proper selection and compaction of the pipe bedding and cover materials is essential. This should be done in accordance with the Standard Specification for Sewer and Water Construction. Bedding material exhibiting a well-defined moisture density relationship should be compacted to 95



percent of ASTM D-698 (Standard proctor). Pipelines and associated manholes along the project route bearing upon suitable soils or upon properly placed and compacted structural fill can be designed to exert a net allowable bearing pressure of 3,000 to 4,000 pounds per square foot dependent upon location and bearing elevation. The use of undercutting, in conjunction with a coarse stone working mat, may be necessary to achieve a suitable bearing subgrade, in at least some areas.

Conventional Trench Backfilling

Backfilling of the pipes and, trenches, should be performed in accordance with the applicable chapters of the Standard Specification for Sewer and Water Construction. This will be dependent on the type of pipe selected, embedment depth and other factors. In general, it is recommended that well-graded granular soils such as those specified in Chapter 6.43.0 of the Standard Specification for Sewer and Water Construction be utilized for bedding, cover and backfill in the utility trenches to reduce the potential for settlement of the fill. Clean crushed granular materials may be used for pipe bedding and cover, provided they are properly placed and compacted.

In planned areas of overlying structures (beneath roads, driveways, utilities, and related structures), any fill used above the pipe bedding and cover material must consist of a relatively well-graded granular material (such as portions of the existing base course materials). It is considered extremely difficult to achieve proper compaction of clay and high silt content soils in narrow utility trenches, and they are therefore not recommended for reuse in areas of overlying structures. Such materials can be used in landscape areas, if desired, recognizing that some subsidence may occur following construction. Substantial importing of suitable granular materials may be necessary.

Placement of bedding and cover material shall be sufficient to protect the type of pipe selected as specified by the pipe manufacturer. Trench backfill should be placed in layers not more than 12 inches in loose thickness before compaction, except that the first lift of backfill placed over the pipe can be increased to 24 inches if necessary to protect the pipe from compaction equipment. Subsequently thinner lifts may be required depending on the type and size of compaction equipment available.

It is recommended that mechanical compaction be used to achieve uniform consolidation of all fill materials. Proper moisture control is essential to reduce the amount of compactive effort necessary to achieve the specified density. It is recommended that backfill soils be placed at moisture contents within a few percent of their optimum moisture content.

Each lift of backfill must be compacted to a density of at least 95 percent of the maximum dry density as determined by the Standard Proctor method, ASTM D698, as specified in Section 2.6.14b of the Standard Specification for Sewer and Water Construction.



STORMWATER MANAGEMENT AREA CONSIDERATIONS

As requested by the client tests pits TP-1 and TP-2 were performed in the area of the proposed bioretention basin. The subgrade soils encountered at these locations have been visually classified in general accordance with the USDA textural soil classification system.

The surface materials encountered at the test pit locations consisted of about 4 inches to 1 foot of dark brown silt loam. Beneath the surface materials were natural soils consisting of silty clay loam, silt loam, sandy loam, and loamy sand to the termination depth. Groundwater was not encountered at TP-1 and TP-2.

With regard to the above soil and groundwater conditions encountered at the borings, NR 151.124(4)(c)1 and 2 – *Infiltration rate exemptions* indicates that infiltration practices located in an area where the infiltration rate of the soil measured at the proposed bottom of the infiltration system is less than 0.6 inches per hour using a scientifically credible field test method; or an area where the least permeable soil horizon to 5 feet below the proposed bottom of the infiltration system using the USDA method of soils analysis consists of sandy clay loam, clay loam, silty clay loam, sandy clay, silty clay or clay may be credited toward meeting the requirements, but the decision to infiltrate under these conditions is optional. In addition, NR 151.124(4)(b)1 – *Separation distances* indicates that infiltration practices shall be located so that the characteristics of the soil and the separation distance between the bottom of the infiltration system and the elevation of seasonal high groundwater or the top of bedrock are in accordance with the following Table (reproduced from NR 151.124):

Table 3. Separation Distances and Soil Characteristics		
Source Area	Separation Distance	Soil Characteristics
Industrial, Commercial, Institutional Parking Lots and Roads	5 feet or more	Filtering Layer*
Residential Arterial Roads	5 feet or more	Filtering Layer*
Roofs Draining to Surface Infiltration Practices	1 foot or more	Native or Engineered Soil with Particles Finer than Coarse Sand
Roofs Draining to Surface Infiltration Practices	Not Applicable	
All Other Impervious Source Areas	3 feet or more	Filtering Layer*

*Defined in NR 151.002(14r) as a “soil that has at least a 3-foot deep layer with at least 20 percent fines; or at least a 5-foot deep layer with at least 10 percent fines; or an engineered soil with an equivalent level of protection as determined by the regulatory authority for the site.”

The information shown above is a selected excerpt from NR151 that is intended only as general guidance for considering stormwater management in conjunction with the encountered



subsurface conditions at the borings. Basin design must be performed by a qualified and experienced firm. In addition, the entirety of Chapter NR151 of the Wisconsin Administrative Code, the Site Evaluation for Stormwater Infiltration (1002) document, and other applicable references; along with appropriate state, local or other municipal requirements must be consulted as part of site specific stormwater design.

It is recommended that stormwater management basins not be placed in close proximity to basements or other below grade structures. Proper and careful consideration of soils and subsurface conditions must be given during site and design planning, and extreme care must be exercised during construction. Lateral migration of water may result in substantially increased sump pump activity and can quickly overcome the ability of such pumps to maintain a desirable water level, resulting in significant flooding. The potential for such conditions to occur can greatly increase when below grade floor elevations encroach upon or are below the elevation of basin bottoms and/or when basins are placed in close proximity to structures (strongly not recommended). In addition, the presence of granular or other generally permeable soils, which is typically necessary in the areas of structures for utility backfill, alongside basement walls, or within other development excavations/trenches can act as extensive migration channels to rapidly carry large volumes of water from basins and into nearby below grade structures. Building codes or municipal regulations may require that floor elevations of below grade structures be a specified distance above the water level of nearby basins or other stormwater features. It is therefore recommended that the design engineer (or other appropriate representative) review applicable municipal or other regulatory requirements and verify the design normal and design high water elevations of stormwater basins/features with respect to planned or existing below grade slab elevations.

GENERAL COMMENTS

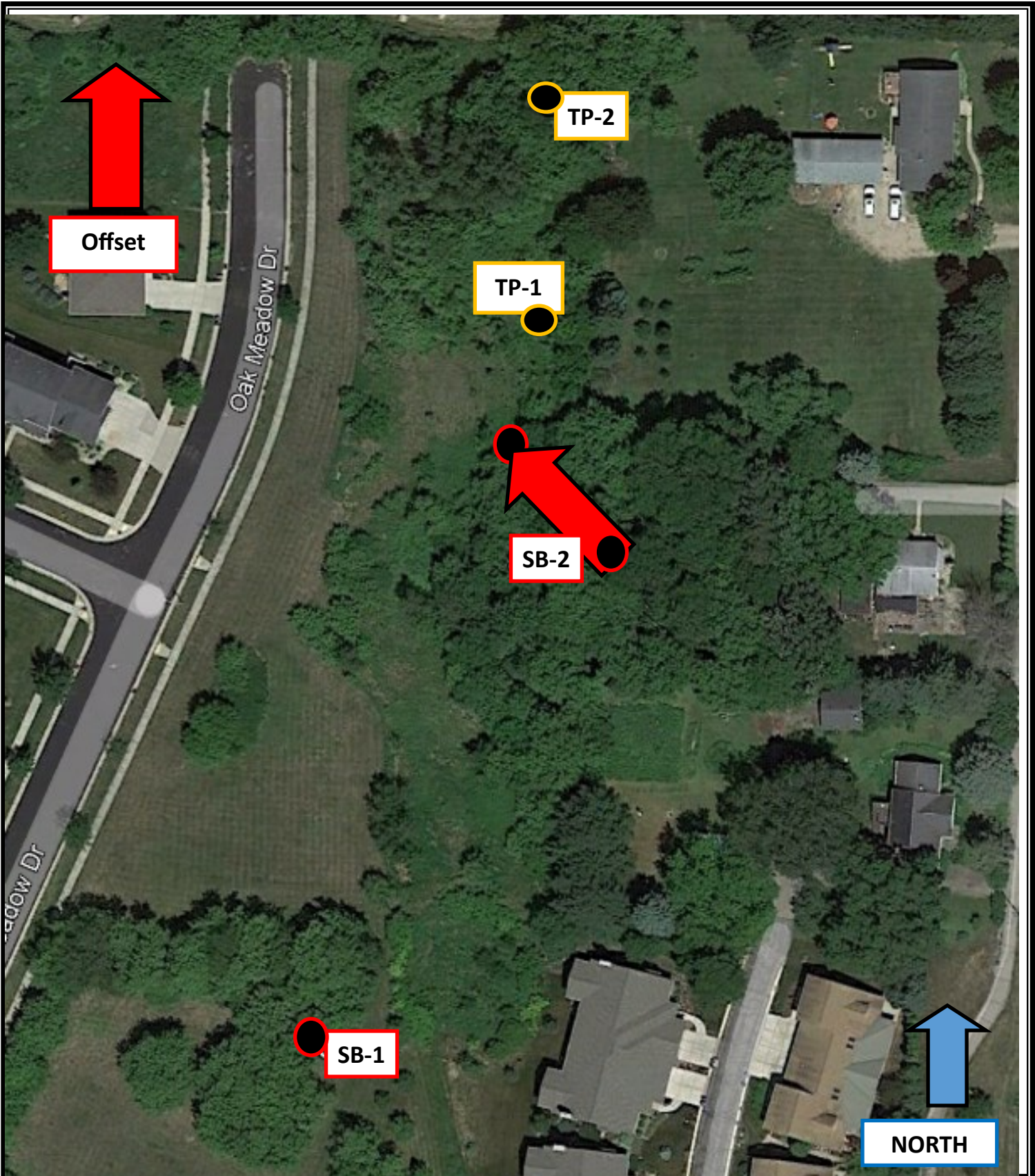
This geotechnical exploration and subgrade evaluation has been prepared to aid in the evaluation of the soil conditions on this site. The recommendations presented herein are based on the available soil information and the preliminary project information provided. Any changes in the planned project activities should be brought to the attention of the soil engineer to determine if modifications in the recommendations are required. The final design plans and specifications should also be reviewed by the soil engineer to determine that the recommendations presented herein have been interpreted and implemented as intended.

This geotechnical study has been conducted in a manner consistent with that level of care ordinarily exercised by members of the profession currently practicing in the same locality under similar conditions. The findings, recommendations and opinions contained herein have been promulgated in accordance with generally accepted practice in the fields of foundation engineering, soils mechanics, and engineering geology. No other representations, expressed or implied, and no warranty or guarantee is included or intended in this report.

It is recommended that the earthwork and foundation operations be monitored by the soil engineer, to test and evaluate the subgrade stability, bearing capacities, and the selection,



placement and compaction of controlled fills. The Wisconsin DOT Standard Specifications for Highway and Structure Construction can also serve as a guide in implementing the subgrade preparation and other earthwork operations.



Oak Meadow Sanitary Sewer and Bioretention Basin
 NEC of Oak Meadow Drive and Targhee Street
 Fitchburg, WI

SCALE: 1 inch = 50 feet (approx.)

DATE: 1/15/2024

BORING/TEST PIT LOCATION PLAN

PSI PROJECT NO. 0052337

DATE STARTED: 12/19/23 **DRILL COMPANY:** PSI, Inc.
DATE COMPLETED: 12/19/23 **DRILLER:** DT **LOGGED BY:** AW
COMPLETION DEPTH: 15.0 ft **DRILL RIG:** ASV D-50 ATV - Rig #420
BENCHMARK: N/A **DRILLING METHOD:** Hollow Stem Auger
ELEVATION: 1020 ft **SAMPLING METHOD:** 2-in SS
LATITUDE: _____ **HAMMER TYPE:** Automatic
LONGITUDE: _____ **EFFICIENCY:** N/A
STATION: N/A **OFFSET:** N/A **REVIEWED BY:** _____
REMARKS: _____

BORING SB-1

Water	▽	While Drilling	Not Obsvd
	▼	Upon Completion	Not Obsvd
	⏸	Delay	N/A

BORING LOCATION: _____

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS)	STANDARD PENETRATION TEST DATA N in blows/ft @	Moisture, %	Strength, tsf	Additional Remarks
0		Topsoil				Topsoil, Dark Brown Lean Clay With Sand, Moist (8"± Thick)	TPSL					
		Brown Lean Clay		1	14	Brown Lean Clay, Trace Sand, Moist	CL	4-5-7 N=12				
		Brown to Reddish Brown Silty Fine Sand With Gravel		2	8	Brown to Reddish Brown Silty Fine Sand With Gravel, Possible Cobbles from 2'-15', Moist		13-17-19 N=36				
1015	5			3	6			11-8-17 N=25				
				4	12		SM	10-14-16 N=30				
1010	10			5	12			12-15-18 N=33				
1005	15					End of Boring at 15'						
						Cave-In at 9'						



Professional Service Industries, Inc.
 821 Corporate Court, Suite 100
 Waukesha, WI 53189
 Telephone: (262) 521-2125

PROJECT NO.: 00523337
PROJECT: Oak Meadow Dr Sewer
LOCATION: Oak Meadow Dr
 Fitchburg, WI

DATE STARTED: 12/19/23 **DRILL COMPANY:** PSI, Inc.
DATE COMPLETED: 12/19/23 **DRILLER:** DT **LOGGED BY:** AW
COMPLETION DEPTH: 15.0 ft **DRILL RIG:** ASV D-50 ATV - Rig #420
BENCHMARK: N/A **DRILLING METHOD:** Hollow Stem Auger
ELEVATION: 1005 ft **SAMPLING METHOD:** 2-in SS
LATITUDE: _____ **HAMMER TYPE:** Automatic
LONGITUDE: _____ **EFFICIENCY:** N/A
STATION: N/A **OFFSET:** N/A **REVIEWED BY:** _____
REMARKS: _____

BORING SB-2

Water	▽	While Drilling	Not Obsvd
	▼	Upon Completion	Not Obsvd
	▽	Delay	N/A

BORING LOCATION: _____

Elevation (feet)	Depth (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @	Additional Remarks
										X Moisture ◻ PL ◉ ◼ LL ▲ Qu * Qp	
1000	0	[Topsoil]				Topsoil, Dark Brown Lean Clay, Moist (8"± Thick)	TPSL	24			
		[Brown Lean Clay]		1	12	Brown Lean Clay With Sand, Moist	CL	3-4-9 N=13			Q _r = 1.6 tsf
		[Brown to Reddish Brown Silty Fine Sand and Gravel]		2	8	Brown to Reddish Brown Silty Fine Sand and Gravel, Moist		5-13-39 N=52			
	5			3	0		SM	27-41-39 N=80			
		[Silty Fine Sand]		4	14			19-19-42 N=61			
995	10			5	14			21-20-39 N=59			
990	15					End of Boring at 15'					
						Cave-In at 8'					
						Boring offset 50' NW due to dense forest					



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PROJECT: Oak Meadow Dr Sewer
LOCATION: Oak Meadow Dr
 Fitchburg, WI

DATE STARTED: 12/19/23 **DRILL COMPANY:** PSI, Inc.
DATE COMPLETED: 12/19/23 **DRILLER:** DT **LOGGED BY:** AW
COMPLETION DEPTH: 15.0 ft **DRILL RIG:** ASV D-50 ATV - Rig #420
BENCHMARK: N/A **DRILLING METHOD:** Hollow Stem Auger
ELEVATION: 992 ft **SAMPLING METHOD:** 2-in SS
LATITUDE: _____ **HAMMER TYPE:** Automatic
LONGITUDE: _____ **EFFICIENCY:** N/A
STATION: N/A **OFFSET:** N/A **REVIEWED BY:** _____
REMARKS: _____

TEST PIT TP-2

Water	▽	While Excavating	Not Obsvd
	▼	Upon Completion	Not Obsvd
	▽	Delay	N/A

BORING LOCATION: _____

Elevation (feet)	Depth, (feet)	Graphic Log	Sample Type	Sample No.	Recovery (inches)	MATERIAL DESCRIPTION	USCS Classification	SPT Blows per 6-inch (SS)	Moisture, %	STANDARD PENETRATION TEST DATA N in blows/ft @	Additional Remarks
										X Moisture ▣ PL ▣ LL 0 25 50	
										STRENGTH, tsf ▲ Qu * Qp 0 2.0 4.0	
	0					Dark Brown Silt Loam					
						Dark Yellowish Brown Silt Loam					
	990					Yellowish Brown Silty Clay Loam					
						Brown Gravelly Sandy Loam					
	5					Pale Brown Gravelly Loamy Sand					
	985					Yellowish Brown Gravelly Sandy Loam					
						Light Reddish Brown Gravelly Sandy Loam (Interbedded Light Yellowish Brown Fine Sand Seams)					
	10										
	980										
	15					End of Boring at 15'					



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 Fitchburg, WI

GENERAL NOTES

SAMPLE IDENTIFICATION

The Unified Soil Classification System (USCS), AASHTO 1988 and ASTM designations D2487 and D-2488 are used to identify the encountered materials unless otherwise noted. Coarse-grained soils are defined as having more than 50% of their dry weight retained on a #200 sieve (0.075mm); they are described as: boulders, cobbles, gravel or sand. Fine-grained soils have less than 50% of their dry weight retained on a #200 sieve; they are defined as silts or clay depending on their Atterberg Limit attributes. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size.

DRILLING AND SAMPLING SYMBOLS

SFA: Solid Flight Auger - typically 4" diameter flights, except where noted.	☒ SS: Split-Spoon - 1 3/8" I.D., 2" O.D., except where noted.
HSA: Hollow Stem Auger - typically 3 1/4" or 4 1/4" I.D. openings, except where noted.	■ ST: Shelby Tube - 3" O.D., except where noted.
M.R.: Mud Rotary - Uses a rotary head with Bentonite or Polymer Slurry	▮ RC: Rock Core
R.C.: Diamond Bit Core Sampler	⬇ TC: Texas Cone
H.A.: Hand Auger	☞ BS: Bulk Sample
P.A.: Power Auger - Handheld motorized auger	☑ PM: Pressuremeter
	CPT-U: Cone Penetrometer Testing with Pore-Pressure Readings

SOIL PROPERTY SYMBOLS

- N: Standard "N" penetration: Blows per foot of a 140 pound hammer falling 30 inches on a 2-inch O.D. Split-Spoon.
- N₆₀: A "N" penetration value corrected to an equivalent 60% hammer energy transfer efficiency (ETR)
- Q_u: Unconfined compressive strength, TSF
- Q_p: Pocket penetrometer value, unconfined compressive strength, TSF
- w%: Moisture/water content, %
- LL: Liquid Limit, %
- PL: Plastic Limit, %
- PI: Plasticity Index = (LL-PL), %
- DD: Dry unit weight, pcf
- ▼, ▼, ▼ Apparent groundwater level at time noted

RELATIVE DENSITY OF COARSE-GRAINED SOILS

<u>Relative Density</u>	<u>N - Blows/foot</u>
Very Loose	0 - 4
Loose	4 - 10
Medium Dense	10 - 30
Dense	30 - 50
Very Dense	50 - 80
Extremely Dense	80+

ANGULARITY OF COARSE-GRAINED PARTICLES

<u>Description</u>	<u>Criteria</u>
Angular:	Particles have sharp edges and relatively plane sides with unpolished surfaces
Subangular:	Particles are similar to angular description, but have rounded edges
Subrounded:	Particles have nearly plane sides, but have well-rounded corners and edges
Rounded:	Particles have smoothly curved sides and no edges

GRAIN-SIZE TERMINOLOGY

<u>Component</u>	<u>Size Range</u>
Boulders:	Over 300 mm (>12 in.)
Cobbles:	75 mm to 300 mm (3 in. to 12 in.)
Coarse-Grained Gravel:	19 mm to 75 mm (¾ in. to 3 in.)
Fine-Grained Gravel:	4.75 mm to 19 mm (No.4 to ¾ in.)
Coarse-Grained Sand:	2 mm to 4.75 mm (No.10 to No.4)
Medium-Grained Sand:	0.42 mm to 2 mm (No.40 to No.10)
Fine-Grained Sand:	0.075 mm to 0.42 mm (No. 200 to No.40)
Silt:	0.005 mm to 0.075 mm
Clay:	<0.005 mm

PARTICLE SHAPE

<u>Description</u>	<u>Criteria</u>
Flat:	Particles with width/thickness ratio > 3
Elongated:	Particles with length/width ratio > 3
Flat & Elongated:	Particles meet criteria for both flat and elongated

RELATIVE PROPORTIONS OF FINES

<u>Descriptive Term</u>	<u>% Dry Weight</u>
Trace:	< 5%
With:	5% to 12%
Modifier:	>12%

GENERAL NOTES

(Continued)

CONSISTENCY OF FINE-GRAINED SOILS

<u>Q_u - TSF</u>	<u>N - Blows/foot</u>	<u>Consistency</u>
0 - 0.25	0 - 2	Very Soft
0.25 - 0.50	2 - 4	Soft
0.50 - 1.00	4 - 8	Firm (Medium Stiff)
1.00 - 2.00	8 - 15	Stiff
2.00 - 4.00	15 - 30	Very Stiff
4.00 - 8.00	30 - 50	Hard
8.00+	50+	Very Hard

MOISTURE CONDITION DESCRIPTION

<u>Description</u>	<u>Criteria</u>
Dry:	Absence of moisture, dusty, dry to the touch
Moist:	Damp but no visible water
Wet:	Visible free water, usually soil is below water table

RELATIVE PROPORTIONS OF SAND AND GRAVEL

<u>Descriptive Term</u>	<u>% Dry Weight</u>
Trace:	< 15%
With:	15% to 30%
Modifier:	>30%

STRUCTURE DESCRIPTION

<u>Description</u>	<u>Criteria</u>	<u>Description</u>	<u>Criteria</u>
Stratified:	Alternating layers of varying material or color with layers at least ¼-inch (6 mm) thick	Blocky:	Cohesive soil that can be broken down into small angular lumps which resist further breakdown
Laminated:	Alternating layers of varying material or color with layers less than ¼-inch (6 mm) thick	Lensed:	Inclusion of small pockets of different soils
Fissured:	Breaks along definite planes of fracture with little resistance to fracturing	Layer:	Inclusion greater than 3 inches thick (75 mm)
Slickensided:	Fracture planes appear polished or glossy, sometimes striated	Seam:	Inclusion 1/8-inch to 3 inches (3 to 75 mm) thick extending through the sample
		Parting:	Inclusion less than 1/8-inch (3 mm) thick

SCALE OF RELATIVE ROCK HARDNESS

<u>Q_u - TSF</u>	<u>Consistency</u>
2.5 - 10	Extremely Soft
10 - 50	Very Soft
50 - 250	Soft
250 - 525	Medium Hard
525 - 1,050	Moderately Hard
1,050 - 2,600	Hard
>2,600	Very Hard

ROCK BEDDING THICKNESSES

<u>Description</u>	<u>Criteria</u>
Very Thick Bedded	Greater than 3-foot (>1.0 m)
Thick Bedded	1-foot to 3-foot (0.3 m to 1.0 m)
Medium Bedded	4-inch to 1-foot (0.1 m to 0.3 m)
Thin Bedded	1¼-inch to 4-inch (30 mm to 100 mm)
Very Thin Bedded	½-inch to 1¼-inch (10 mm to 30 mm)
Thickly Laminated	1/8-inch to ½-inch (3 mm to 10 mm)
Thinly Laminated	1/8-inch or less "paper thin" (<3 mm)

ROCK VOIDS

<u>Voids</u>	<u>Void Diameter</u>
Pit	<6 mm (<0.25 in)
Vug	6 mm to 50 mm (0.25 in to 2 in)
Cavity	50 mm to 600 mm (2 in to 24 in)
Cave	>600 mm (>24 in)

GRAIN-SIZED TERMINOLOGY

(Typically Sedimentary Rock)

<u>Component</u>	<u>Size Range</u>
Very Coarse Grained	>4.76 mm
Coarse Grained	2.0 mm - 4.76 mm
Medium Grained	0.42 mm - 2.0 mm
Fine Grained	0.075 mm - 0.42 mm
Very Fine Grained	<0.075 mm

ROCK QUALITY DESCRIPTION

<u>Rock Mass Description</u>	<u>RQD Value</u>
Excellent	90 - 100
Good	75 - 90
Fair	50 - 75
Poor	25 - 50
Very Poor	Less than 25

DEGREE OF WEATHERING

Slightly Weathered:	Rock generally fresh, joints stained and discoloration extends into rock up to 25 mm (1 in), open joints may contain clay, core rings under hammer impact.
Weathered:	Rock mass is decomposed 50% or less, significant portions of the rock show discoloration and weathering effects, cores cannot be broken by hand or scraped by knife.
Highly Weathered:	Rock mass is more than 50% decomposed, complete discoloration of rock fabric, core may be extremely broken and gives clunk sound when struck by hammer, may be shaved with a knife.

SOIL CLASSIFICATION CHART

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS	
			GRAPH	LETTER		
COARSE GRAINED SOILS MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVEL AND GRAVELLY SOILS (LITTLE OR NO FINES)	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
				GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
		GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES	
	MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)			GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
		CLEAN SANDS			SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
		(LITTLE OR NO FINES)			SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	SANDS WITH FINES			SM	SILTY SANDS, SAND - SILT MIXTURES
		(APPRECIABLE AMOUNT OF FINES)			SC	CLAYEY SANDS, SAND - CLAY MIXTURES
					ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
	FINE GRAINED SOILS MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS LIQUID LIMIT LESS THAN 50			CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY	
				MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS	
SILTS AND CLAYS LIQUID LIMIT GREATER THAN 50				CH	INORGANIC CLAYS OF HIGH PLASTICITY	
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS	
				PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS	
HIGHLY ORGANIC SOILS						

SOIL EVALUATION - STORM

In accordance with SPS 382.365 & 385, Wis. Adm. Code and WDNR Standard 1002


<p>Attach complete site plan on paper not less than 8 1/2 x 11 inches in size. Plan must include, but not limited to: vertical and horizontal reference point (BM), direction and percent slope, scale or dimensions, north arrow, and BM referenced to nearest road.</p> <p style="text-align: center;">Please print all information.</p> <p>Personal information you provide may be used for secondary purposes [Privacy Law, s. 15.04 (1) (m)].</p>	County
	Dane
	Parcel I.D.
	Reviewed by:
	Date:

Property Owner	Property Location: Oak Meadow Drive, Fitchburg, WI		
	Govt. Lot		
Property Owner's Mailing Address	Lot #	Block #	Subd. Name or CSM#
City State Zip Code Phone Number	<input checked="" type="checkbox"/> City	<input type="checkbox"/> Village	<input type="checkbox"/> Town Nearest Road
	Fitchburg		Oak Meadow Drive

Drainage area _____ <input type="checkbox"/> sq. ft. <input type="checkbox"/> acres Optional: Test Site Suitable for (check all that apply) <input type="checkbox"/> Irrigation <input type="checkbox"/> Bioretention trench <input type="checkbox"/> Trench(es) <input type="checkbox"/> Rain Garden <input type="checkbox"/> Grassed swale <input type="checkbox"/> Reuse <input type="checkbox"/> Infiltration trench <input type="checkbox"/> SDS (> 15' wide) <input type="checkbox"/> Other _____	Hydraulic Application Test Method: <input checked="" type="checkbox"/> Morphological Evaluation <input type="checkbox"/> Double Ring Infiltrometer <input type="checkbox"/> Other (specify)	Soil Moisture Date of Test Pits: December 22, 2023 USDA-NRCS WETS Value: 9 <input checked="" type="checkbox"/> Dry = 1; <input type="checkbox"/> Normal = 2; <input type="checkbox"/> Wet = 3.
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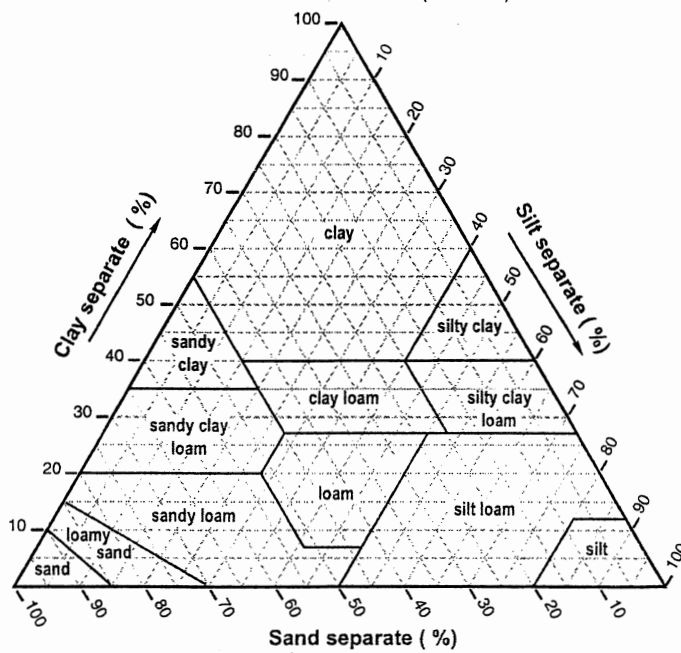
1	Obs. #	<input type="checkbox"/> Boring	TP-1	Ground surface elevation ±				Elevation of limiting factor: >15'			
		<input checked="" type="checkbox"/> Pit									
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic App. Rate Inches/Hr.	
1	0-4	10YR 3/3		sil	0 cr	mfr		<15		0.13	
2	4-12	10YR 4/6		grl	0 cr	mfr		>15		0.24	
3	12-30	10YR 5/4		grfs	0 m	mvfr		>15		1.63	
4	30-180	2.5YR 6/4		grsl	1 thin pl	mfr		>15		0.5	
Comment:											

2	Obs. #	<input type="checkbox"/> Boring	TP-2	Ground surface elevation ±				Elevation of limiting factor: >15'			
		<input checked="" type="checkbox"/> Pit									
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic App. Rate Inches/Hr.	
1	0-12	10YR 3/3		sil	0 cr	mfr		<15		0.13	
2	12-24	10YR 4/6		sil	1 thin pl	mfr		<15		0.13	
3	24-48	10YR 5/4		sicl	1 thin pl	mfi		<15		0.04	
4	48-72	10YR 5/3		grsl	0 m	mvfr		>15		0.5	
5	72-96	10YR 6/3		grls	0 m	mvfr		>15		1.63	
6	96-120	10YR 5/4		grsl	1 thin pl	mfr		>15		0.5	
7	120-180	2.5YR 6/4		grsl ⁽¹⁾	1 thin pl	mfr		>15		0.5	
Comment: 1. thin seams of 10YR 6/4 Fine Sand at 14 feet											

CST/PSS Name (Please Print)	Signature	CST/PSS/Geologist Number
Patrick J. Patterson		G-229
Address	Date Evaluation Conducted	Telephone Number
821 Corporate Court, Waukesha, WI 53189	12/22/2023	262 521 2125

Texture Triangle:

Fine Earth Texture Classes (———)



TEXTURE MODIFIERS - Conventions for using "Rock Fragment Texture Modifiers" and for using textural adjectives that convey the "% volume" ranges for Rock Fragments - Size and Quantity.

Fragment Content % By Volume	Rock Fragment Modifier Usage
< 15	No texture adjective is used (noun only; e.g., <i>loam</i>).
15 to < 35	Use adjective for appropriate size; e.g., <i>gravelly</i> .
35 to < 60	Use "very" with the appropriate size adjective; e.g., <i>very gravelly</i> .
60 to < 90	Use "extremely" with the appropriate size adjective; e.g., <i>extremely gravelly</i> .
≥ 90	No adjective or modifier. If ≤ 10% fine earth, use the appropriate noun for the dominant size class; e.g., <i>gravel</i> . Use Terms in Lieu of Texture .

(SOIL) TEXTURE

This is the numerical proportion (percent by weight) of sand, silt, and clay in a soil. Sand, silt, and clay content is estimated in the field by hand (or quantitatively measured in the office/lab by hydrometer or pipette) and then placed within the texture triangle to determine **Texture Class**. Estimate the **Texture Class**; e.g., *sandy loam*; or **Subclass**; e.g., *fine sandy loam* of the fine earth (≤ 2 mm) fraction, or choose a **Term in Lieu of Texture**; e.g., *gravel*. If appropriate, use a **Textural Class Modifier**; e.g., *gravelly silt loam*.

NOTE: **Soil Texture** encompasses only the fine earth fraction (≤ 2 mm). **Particle Size Distribution (PSD)** encompasses the whole soil, including both the fine earth fraction (≤ 2 mm; weight %) and rock fragments (> 2 mm; volume %).

TEXTURE CLASS

Texture Class or Subclass	Code	
	Conv.	NASIS
Coarse Sand	cos	COS
Sand	s	S
Fine Sand	fs	FS
Very Fine Sand	vfs	VFS
Loamy Coarse Sand	lcos	LCOS
Loamy Sand	ls	LS
Loamy Fine Sand	lfs	LFS
Loamy Very Fine Sand	lvfs	LVFS
Coarse Sandy Loam	cosl	COSL
Sandy Loam	sl	SL
Fine Sandy Loam	fsl	FSL
Very Fine Sandy Loam	vfsl	VFSL
Loam	l	L
Silt Loam	sil	SIL
Silt	si	SI
Sandy Clay Loam	scl	SCL
Clay Loam	cl	CL
Silty Clay Loam	sicl	SICL
Sandy Clay	sc	SC
Silty Clay	sic	SIC
Clay	c	C

TEXTURE MODIFIERS - (adjectives)

ROCK FRAGMENTS: Size & Quantity ¹	Code		Criteria: Percent (By Volume) of Total Rock Fragments and Dominated By (name size): ¹
	Conv.	PDP/ NASIS	
ROCK FRAGMENTS (> 2 mm; ≥ Strongly Cemented)			
Gravelly	GR	GR	≥ 15% but < 35% gravel
Fine Gravelly	FGR	GRF	≥15% but < 35% fine gravel
Medium Gravelly	MGR	GRM	≥15% but < 35% med. gravel
Coarse Gravelly	CGR	GRC	≥ 15% but < 35% coarse gravel
Very Gravelly	VGR	GRV	≥ 35% but < 60% gravel
Extremely Gravelly	XGR	GRX	≥ 60% but < 90% gravel
Cobbly	CB	CB	≥ 15% but < 35% cobbles
Very Cobbly	VCB	CBV	≥ 35% but < 60% cobbles
Extremely Cobbly	XCB	CBX	≥ 60% but < 90% cobbles
Stony	ST	ST	≥ 15% but < 35% stones
Very Stony	VST	STV	≥ 35% but < 60% stones
Extremely Stony	XST	STX	≥ 60% but < 90% stones
Bouldery	BY	BY	≥ 15% but < 35% boulders
Very Bouldery	VBY	BYV	≥ 35% but < 60% boulders
Extremely Bouldery	XBY	BYX	≥ 60% but < 90% boulders
Channery	CN	CN	≥ 15% but < 35% channers
Very Channery	VCN	CNV	≥ 35% but < 60% channers
Extremely Channery	XCN	CNX	≥ 60% but < 90% channers
Flaggy	FL	FL	≥ 15% but < 35% flagstones
Very Flaggy	VFL	FLV	≥ 35% but < 60% flagstones
Extremely Flaggy	XFL	FLX	≥ 60% but < 90% flagstones
PARAROCK FRAGMENTS (> 2 mm; < Strongly Cemented) ^{2, 3}			
Parabouldery	PBY	PBY	(same criteria as bouldery)
Very Parabouldery	VPBY	PBYV	(same criteria as very bouldery)
Extr. Parabouldery	XPBY	PBYX	(same criteria as ext. bouldery)
etc.	etc.	etc.	(same criteria as non-para)

¹ The "Quantity" modifier (e.g., *very*) is based on the total rock fragment content. The "Size" modifier (e.g., *cobbly*) is independently based on the largest, dominant fragment size. For a mixture of sizes (e.g., *gravel and stones*), a smaller size-class is named only if its quantity (%) sufficiently exceeds that of a larger size-class. For field texture determination, a smaller size-class must exceed 2 times the quantity (vol. %) of a larger size class before it is named (e.g., 30% gravel and 14% stones = *very gravelly*, but 20% gravel and 14% stones = *stony*). For more explicit naming criteria see NSSH-Part 618, Exhibit 618.11(Soil Survey Staff, 2001b).



December 21, 2023

Attn: Mr. John Thompson
Thompson Engineering LLC
918 Deming Way, Floor 3
Madison, WI 53717

Re: Tree Preservation Recommendations for the Oak Meadow Drive RePlat

Mr. Thompson

We provided an ISA Certified Arborist on December 14th and 15th, 2023 to complete the requested tree survey at 2614 Oak Meadow Drive. While on site, we surveyed trees 6"+ DBH, and collected the following information:

- Location (GPS, x/y coordinates. 3 feet accuracy or greater)
- Species (Common Name, Scientific name)
- DBH (rounded to the nearest inch)
- Condition Class (Excellent, Good, Fair, Poor, Dead)

Our completed tree inventory data files and site maps are being provided as attachments to this letter.

I have reviewed the requirements of a tree preservation plan outlined within Section 54.50 of the City of Fitchburg Municipal Code. The trees that we have surveyed and assessed do not meet the definition of "desirable trees" that require preservation within this section of the code. As such, I do not believe preservation measures are warranted for the trees surveyed on this site.

Please contact me with any questions.

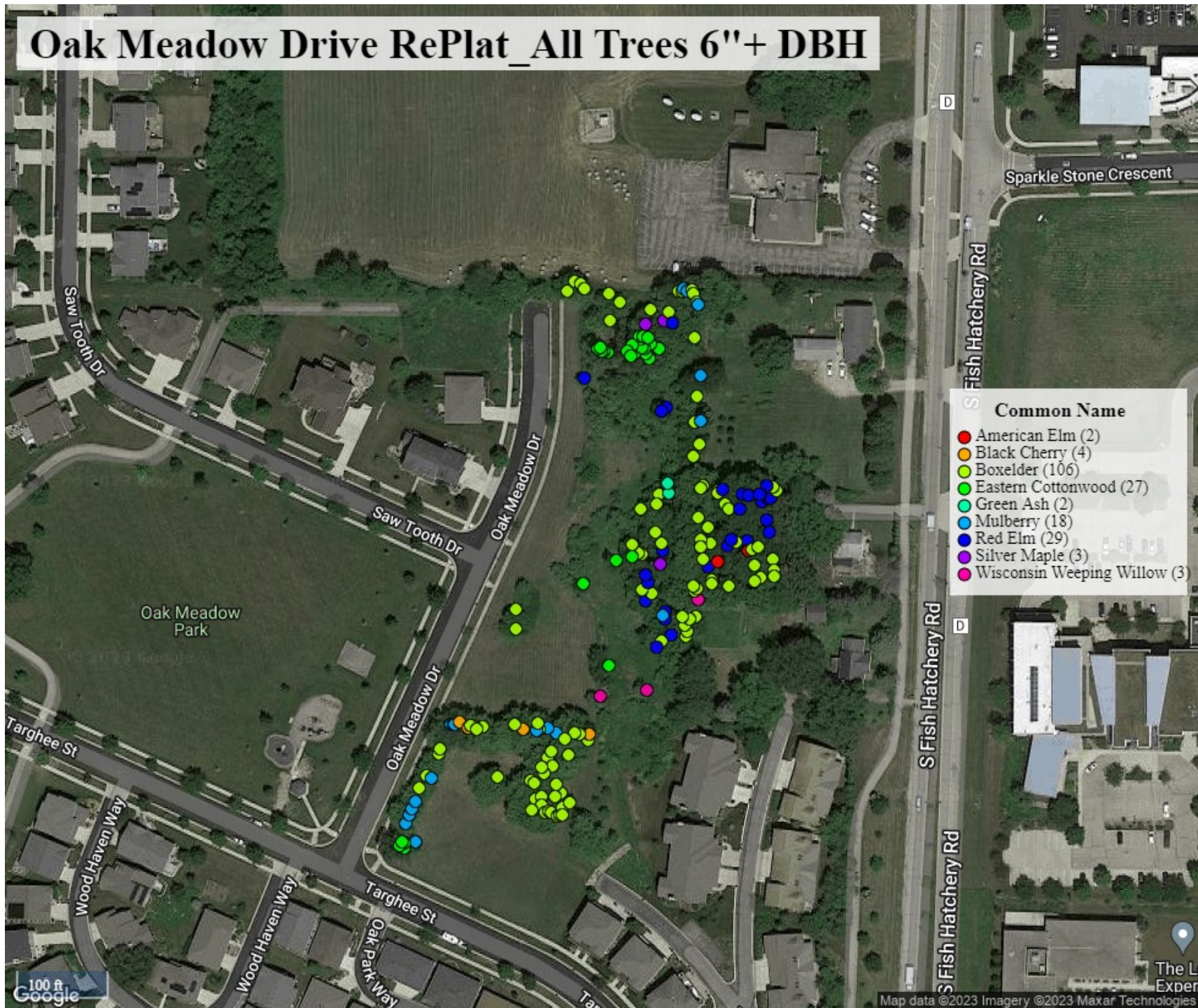
Respectfully submitted,

A handwritten signature in black ink, appearing to read 'Nathan Schuettepelz', is written over a light blue horizontal line.

Nathan Schuettepelz, RCA #732
Consulting Department Manager
ISA Certified Arborist Municipal Specialist # WI-0887AM

Encl.

Oak Meadow Drive RePlat_All Trees 6"+ DBH





THOMPSON ENGINEERING LLC
918 DEMING WAY, FLOOR 3
MADISON, WI 53717

**FINAL PLAT OF OAK MEADOW RESERVE
LEGAL DESCRIPTION**

BEING A PART OF LOT 115, FIRST ADDITION TO OAK MEADOW AS RECORDED IN VOLUME 58-068B OF PLATS ON PAGE 355, AS DOCUMENT NUMBER 4014340, DANE COUNTY REGISTRY, LOCATED IN THE NE1/4-NE1/4 AND THE SE1/4-NE1/4, ALL IN SECTION 16, TOWNSHIP 06 NORTH, RANGE 09 EAST, CITY OF FITCHBURG, DANE COUNTY, WISCONSIN, MORE FULLY DESCRIBED AS FOLLOWS:

COMMENCING AT THE NORTHEAST CORNER OF SAID SECTION 16; THENCE N89°24'37"W ALONG THE NORTH LINE OF THE NORTHEAST QUARTER (NE1/4) OF SAID SECTION 16, 734.39 FEET; THENCE S00°35'13"W, 989.57 FEET TO THE NORTHEAST CORNER OF SAID LOT 115, FIRST ADDITION TO OAK MEADOW AND THE POINT OF BEGINNING THENCE ALONG THE BOUNDARY OF SAID LOT 115 FOR THE NEXT 5 COURSES; 1-THENCE S02°34'28"W, 231.43 FEET; 2-THENCE S88°58'54"E, 92.67 FEET; 3-THENCE S02°31'40"W, 121.71 FEET; 4-THENCE N88°58'54"W, 92.41 FEET; 5-THENCE S02°34'28"W, 146.05 FEET TO A NORTHERLY CORNER OF NOBEL RIDGE CONDOMINIUM HOMES FIRST ADDENDUM, AS RECORDED IN VOLUME 7-075B OF CONDOMINIUM PLATS, ON PAGES 1-7, AS DOCUMENT NUMBER 5448308, DANE COUNTY REGISTRY; THENCE ALONG THE BOUNDARY OF SAID CONDOMINIUM FOR THE NEXT FOUR (4) COURSES; 1-THENCE N65°18'59"W, 103.14 FEET; 2-THENCE S24°41'01"W, 113.91 FEET; 3-THENCE S06°10'08"E, 151.07 FEET; 4-THENCE S29°34'15"W, 63.43 FEET TO A POINT ON THE NORTHEASTERLY RIGHT-OF-WAY LINE OF TARGHEE STREET, SAID POINT ALSO BEING A POINT OF NON-TANGENTIAL CURVATURE; THENCE 30.96 FEET ALONG SAID RIGHT-OF-WAY AND THE ARC OF A CURVE TO THE LEFT, HAVING A RADIUS OF 363.00 FEET, A CENTRAL ANGLE OF 04°53'11", AND A CHORD BEARING N62°52'20"W, 30.95 FEET; THENCE N65°18'56"W ALONG SAID RIGHT-OF-WAY, 184.41 FEET TO A POINT OF CURVATURE; THENCE 39.27 FEET ALONG SAID RIGHT-OF-WAY AND THE ARC OF A CURVE TO THE RIGHT, HAVING A RADIUS OF 25.00 FEET, A CENTRAL ANGLE OF 90°00'00", AND A CHORD BEARING N20°18'56"W, 35.36 FEET TO THE SOUTHEASTERLY RIGHT-OF-WAY OF OAK MEADOW DRIVE; THENCE N24°41'04"E ALONG SAID SOUTHEASTERLY RIGHT-OF-WAY, 424.58 FEET TO A POINT OF CURVATURE; THENCE 165.09 FEET ALONG SAID RIGHT-OF-WAY AND THE ARC OF A CURVE TO THE LEFT, HAVING A RADIUS OF 388.00 FEET, A CENTRAL ANGLE OF 24°22'44", AND A CHORD BEARING N12°29'42"E, 163.85 FEET; THENCE N00°18'20"E ALONG SAID RIGHT-OF-WAY, 96.24 FEET TO THE NORTHWEST CORNER OF SAID LOT 115; THENCE S89°24'24"E ALONG THE NORTH LINE OF SAID LOT 115, 172.52 FEET TO THE NORTHEAST CORNER OF SAID LOT 115 AND THE POINT OF BEGINNING. SAID DESCRIPTION CONTAINS 160,416 SQ. FT. OR 3.683 ACRES MORE OR LESS..